

PLAIN CITY CORPORATION

PUBLIC WORKS STANDARDS

JUNE 2008  
UPDATED: FEBRUARY 2020

SUBMITTED & RECOMMENDED

PAUL J. TAYLOR P.E. CITY ENGINEER DATE  
J-U-B ENGINEERS, INC.

APPROVALS

JON BEESLEY DATE  
MAYOR

DIANE HIRSCHI DATE  
CITY RECORDER

DANIEL SCHULER DATE  
PUBLIC WORK DIRECTOR



Engineers • Surveyors • Planners



THE  
LANGDON  
GROUP



GATEWAY  
MAPPING  
INC.

OTHER J-U-B COMPANIES

466 North 900 West, Kaysville, Utah 84037

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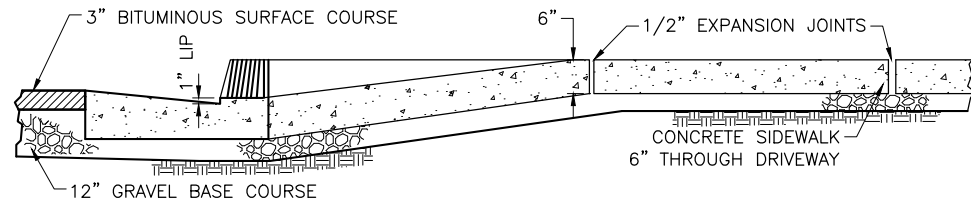
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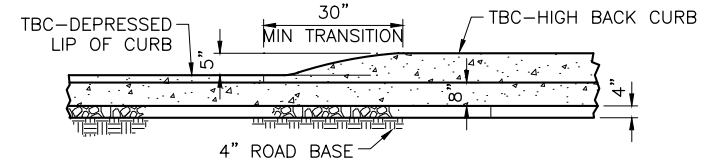
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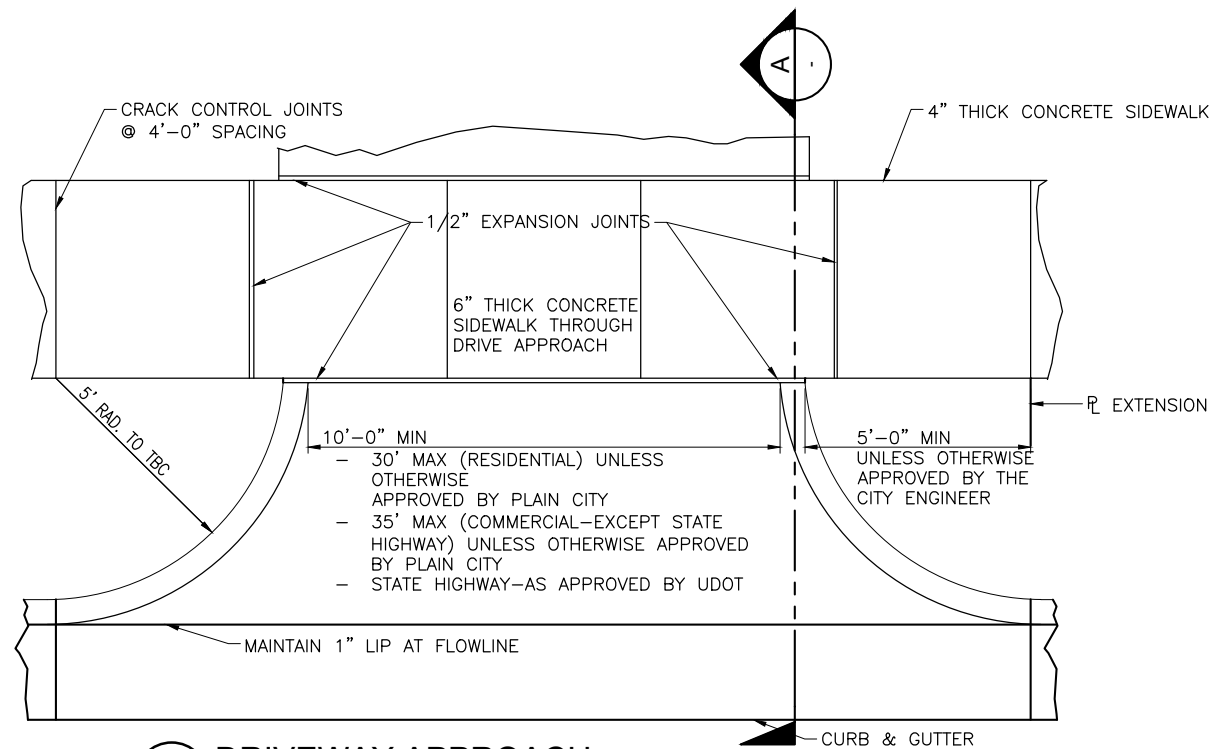




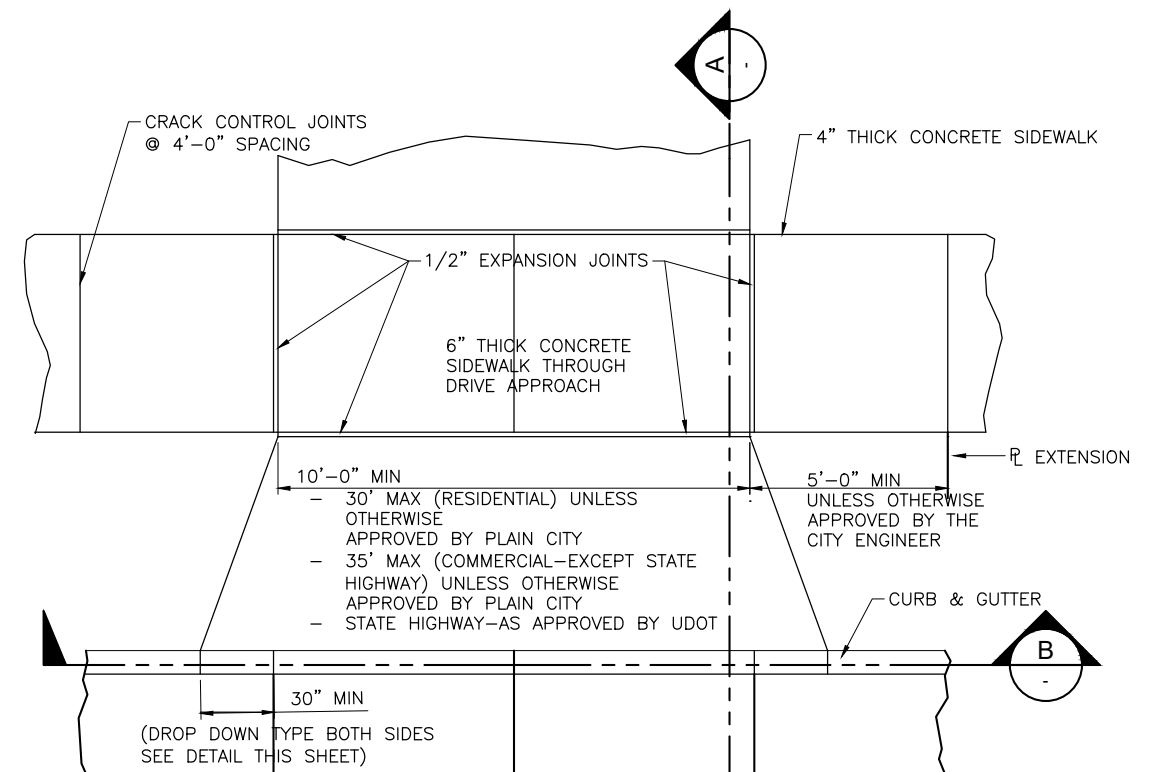
**A SECTION A**  
SCALE: N.T.S.



**B SECTION B**  
SCALE: N.T.S.



**2 DRIVEWAY APPROACH**  
CURB RADIUS STYLE (SPECIAL CASE APPROVAL REQUIRED)  
SCALE: N.T.S.



**3 DRIVEWAY APPROACH**  
DROP DOWN STYLE  
SCALE: N.T.S.

NOTE:  
IN NEW SUBDIVISIONS WHERE FUTURE DRIVEWAY LOCATIONS ARE UNKNOWN,  
THE DRIVEWAY APPROACH SHALL BE MADE BY SAW CUTTING THE BACK OF  
THE EXISTING CURB TO THE REQUIRED DRIVEWAY WIDTH. ALL SAW CUTTING  
SHALL BE ACCOMPLISHED BY A CITY APPROVED LICENSED CONTRACTOR

IF THE SIDEWALK HAS BEEN PREVIOUSLY INSTALLED WHEN THE NEW  
DRIVEWAY IS CONSTRUCTED, THE 4" THICK SIDEWALK SHALL BE REMOVED  
AND REPLACED WITH 6" THICK SIDEWALK THROUGH THE NEW APPROACH,  
AT THE OWNERS OR DEVELOPERS EXPENSE

Plot Date: 4/16/2020 1:41 PM Plotted By: Daniel Johnson  
File Name: C:\Users\jrb\OneDrive\Documents\PROJECTS\PLAIN CITY STANDARDS\2020\CTV STANDARDS.DWG

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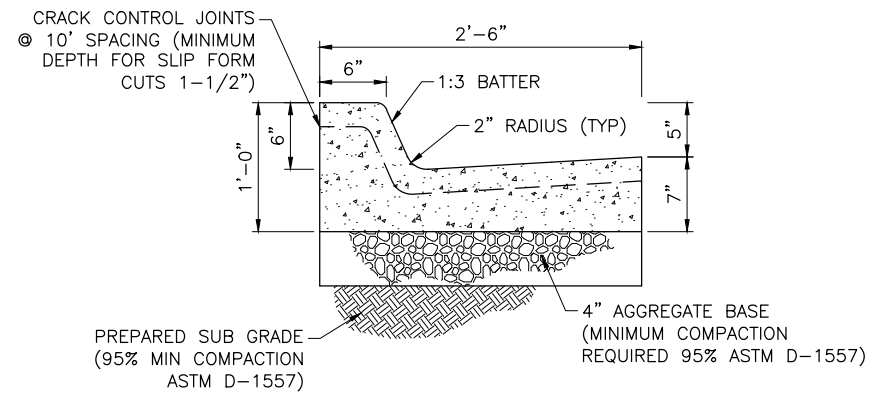
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<b>PUBLIC WORKS STANDARDS PLAIN CITY CORPORATION</b>	
DRIVEWAY APPROACHES	

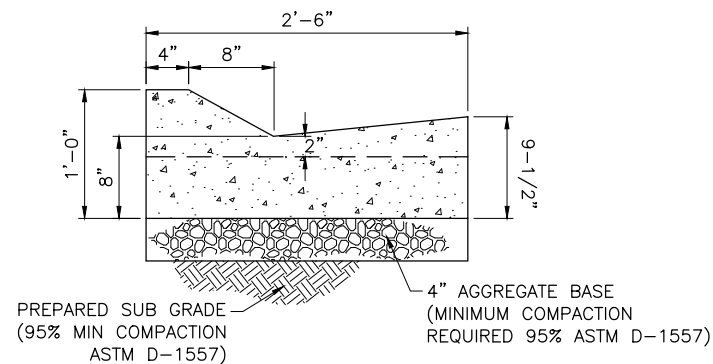
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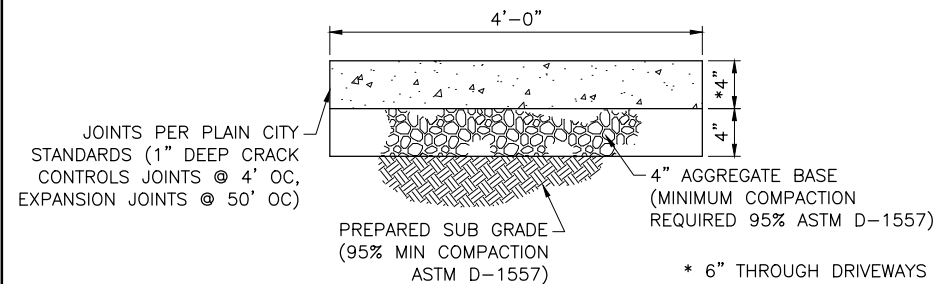




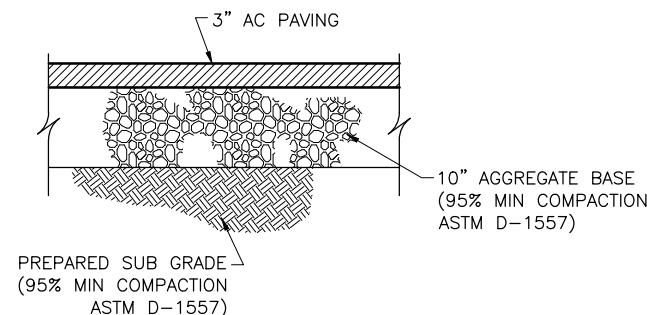
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SCALE: N.T.S.



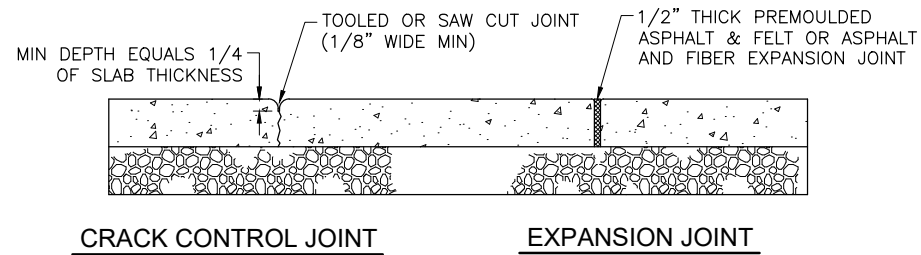
**2** **TYPICAL LOWBACK CURB & GUTTER SECTION**  
SCALE: N.T.S.



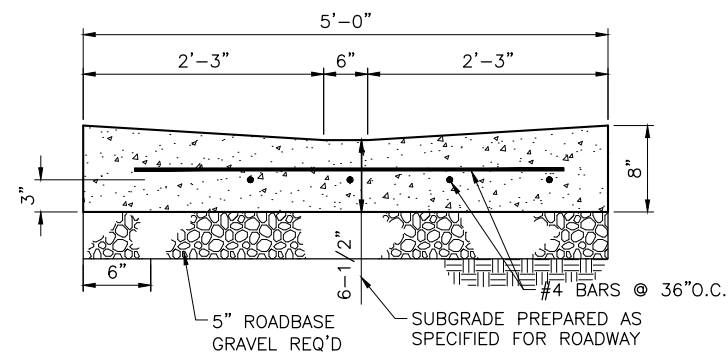
**3** **TYPICAL SIDEWALK**  
SCALE: N.T.S.



**4** **TYPICAL ASPHALT PAVING SECTION**  
SCALE: N.T.S.



**5** **JOINT DETAIL**  
SCALE: N.T.S.



**6** **CROSS DRAIN SECTION**  
SCALE: N.T.S.

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**CURB AND SIDEWALK DETAILS**

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**5**

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6

4" PVC PIPE (LARGER SIZES SHALL BE AS APPROVED BY CITY ENGINEER)

45° PVC STREET ELBOW (BELL x SPIGOT)

SADDLE, SPEC. ROMAC "CB" SEWER SADDLE

EXISTING CONCRETE SEWER PIPE

PVC JOINTS OF RUBBER RING TYPE MUST COMPLY WITH ASTM D-1869

TAPPING INTO EXISTING PIPE & CONNECTING SADDLE TO BE INSPECTED BY THE CITY AND PAID FOR BY THE OWNER, IN ADDITION TO THE SEWER CONNECTION FEE

# 1 TAPPING INTO EXISTING CONCRETE PIPE

## 2 CONNECTING INTO EXISTING WYE OR TEE

### 3 TAPPING INTO EXISTING PVC PIPE

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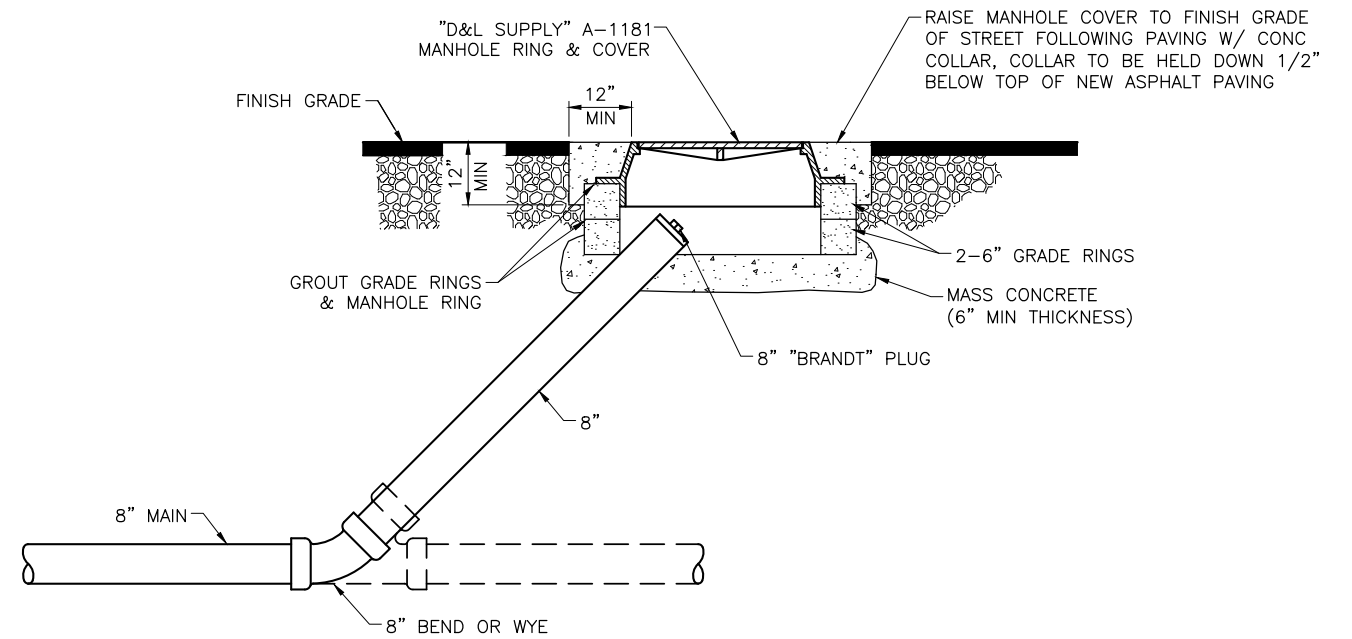
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## SANITARY SEWER CONNECTION DETAILS

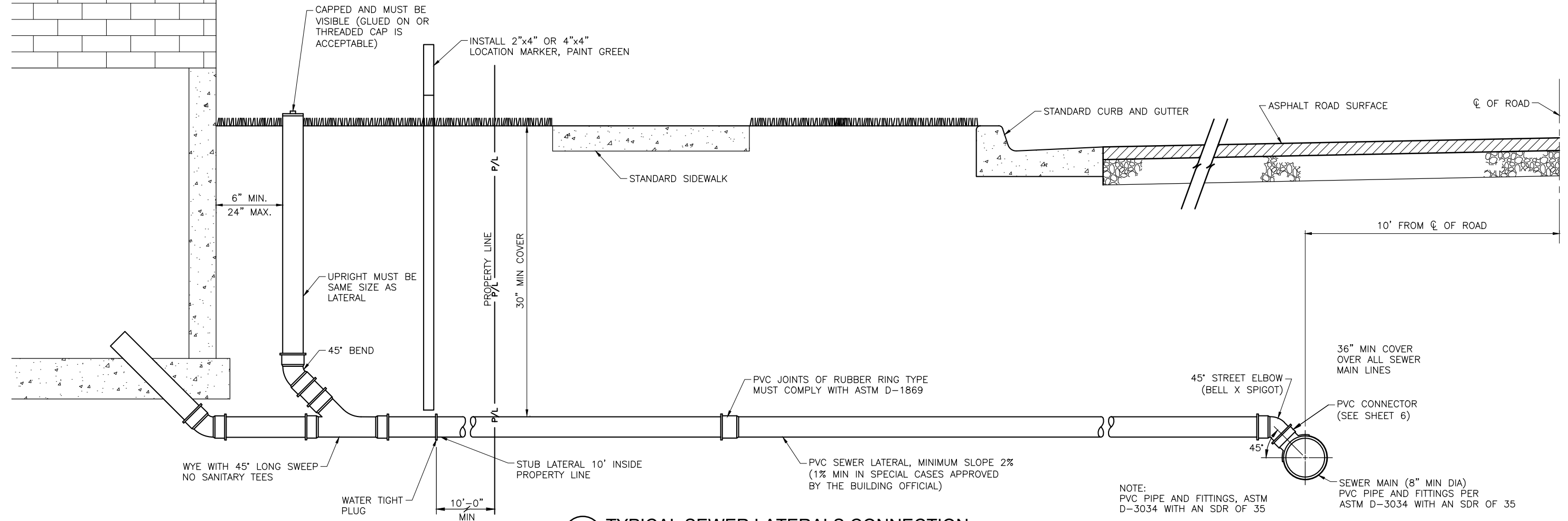
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1. ALL SANITARY SEWER CONNECTIONS ON MAJOR MAINS IN NEW SUBDIVISIONS SHALL BE MADE WITH IN-LINE PRE-FORMED WYES OR TEES UNLESS OTHERWISE APPROVED BY THE CITY ENGINEER
2. FLOWLINE ELEVATION OF LATERALS SHALL EQUAL THE INSIDE TOP OF PIPE ON MAINLINE AT THE CONNECTING POINT
3. SANITARY SEWER PIPE LINES SHALL BE "WHITE" IN COLOR  
SUB-SURFACE DRAIN LINES SHALL BE "GREEN" IN COLOR
4. ALL SANITARY SEWER LINES SHALL BE INSPECTED BY MEANS OF VIDEO CAMERA, FOLLOWING CONSTRUCTION. PRIOR TO VIDEO, PIPE SHALL BE FLUSHED WITH A PRESSURIZED CLEANING TRUCK. ALL WATER & DEBRIS SHALL BE REMOVED FROM THE LOW END MANHOLE. CONNECTIONS TO EXISTING SEWER LINES SHALL BE BLOCKED TO PREVENT FLUSHED DEBRIS FROM ENTERING SEWER. CLEAN WATER SHALL BE ADDED TO LINE PRIOR TO VIDEOING. ALL PROBLEMS FOUND AND CORRECTED TO BE RE-INSPECTED BY THE SAME PROCEDURE VIDEOS TO BE RETAINED BY CITY
5. ALL WORK WITHIN THE CITY'S RIGHT-OF-WAY TO BE PERFORMED BY A LICENSED CONTRACTOR WITH ALL APPLICABLE PERMITS
6. ALL SEWER SERVICE LATERALS SHALL INCLUDE A BACKFLOW PREVENTION DEVICE AS REQUIRED BY THE PLAIN CITY BUILDING OFFICIAL



# 1 DRAIN/SEWER MAINLINE CLEANOUT



## 2 TYPICAL SEWER LATERALS CONNECTION

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Date Created: 2/27/2020 WAVS:JRE	<b>REUSE OF DRAWINGS</b>				
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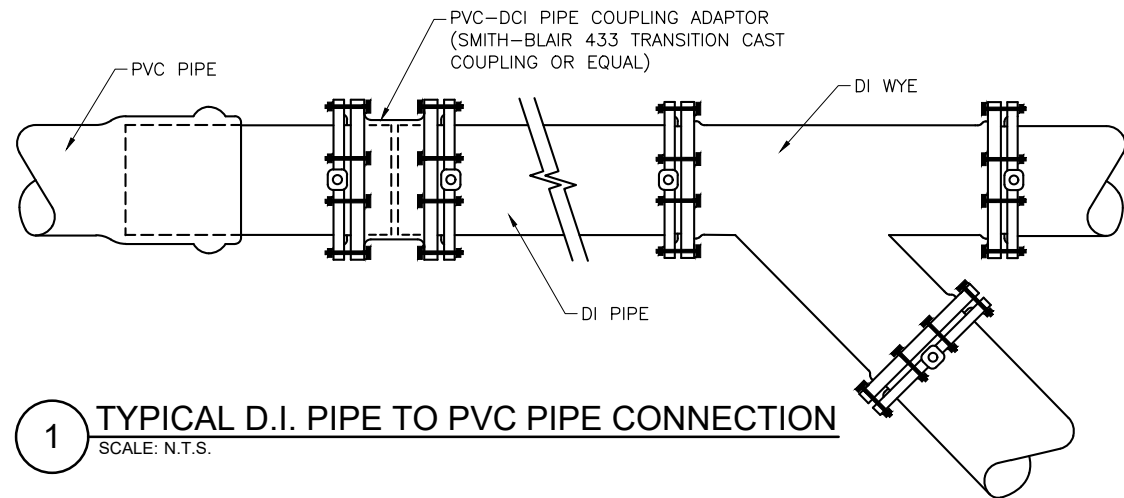
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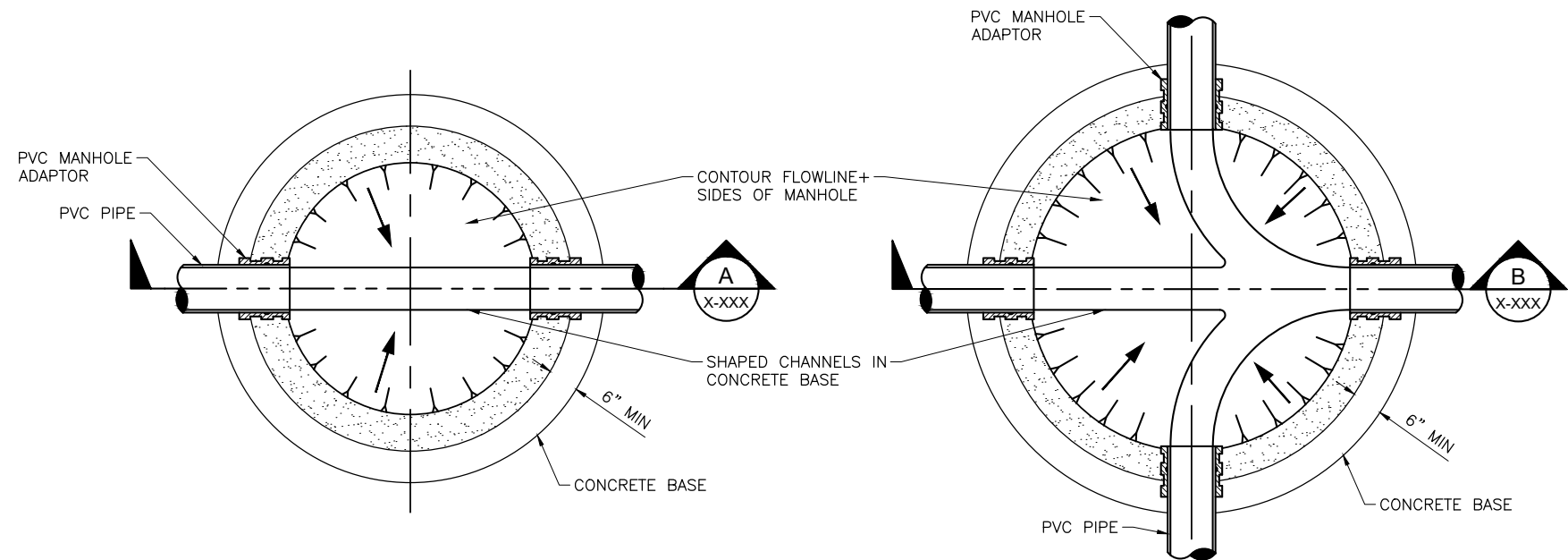
## SANITARY SEWER LATERAL & CLEANOUT DETAILS

LAST UPDATED: 4/16/2020

3

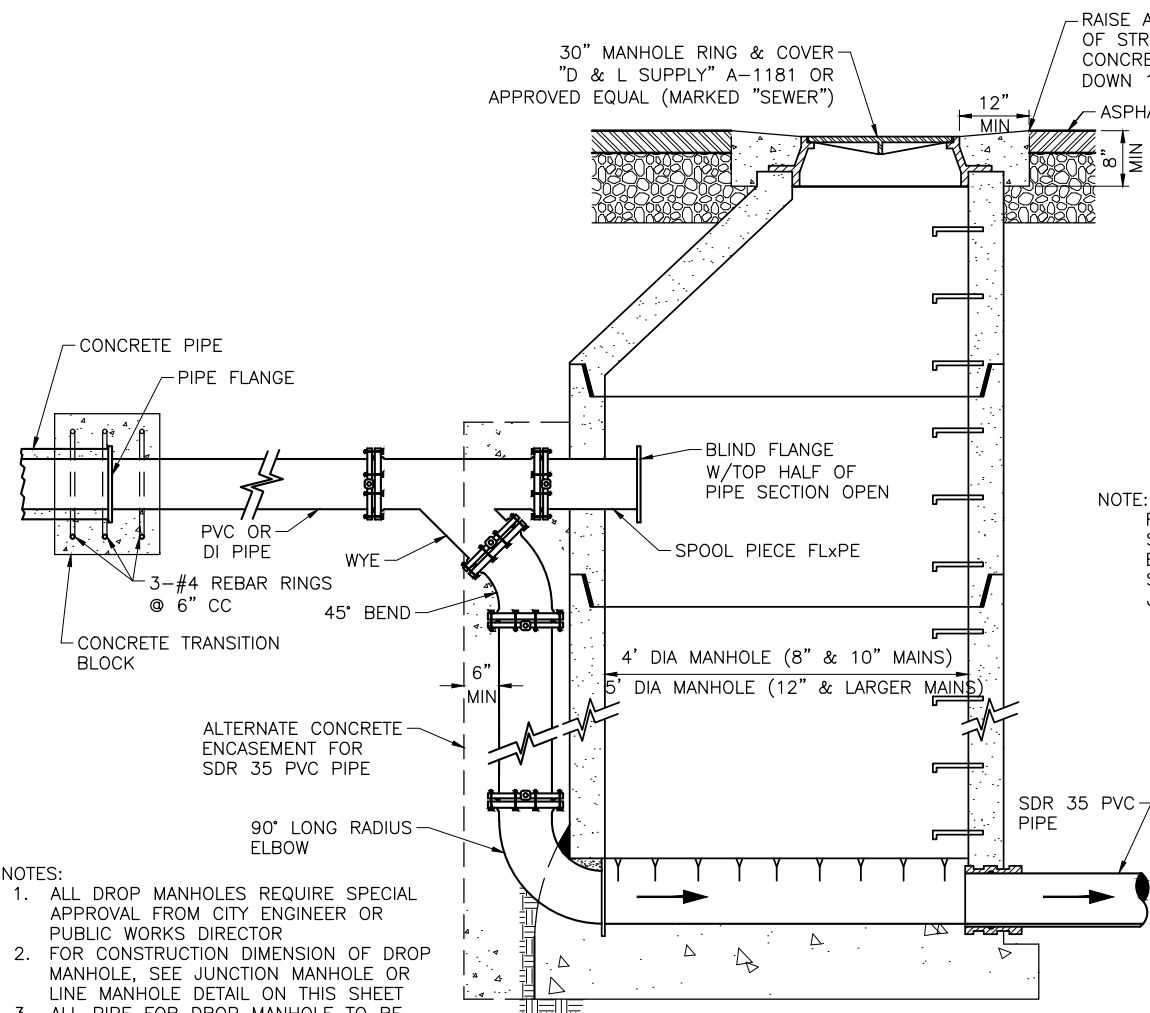


**1 TYPICAL D.I. PIPE TO PVC PIPE CONNECTION**  
SCALE: N.T.S.

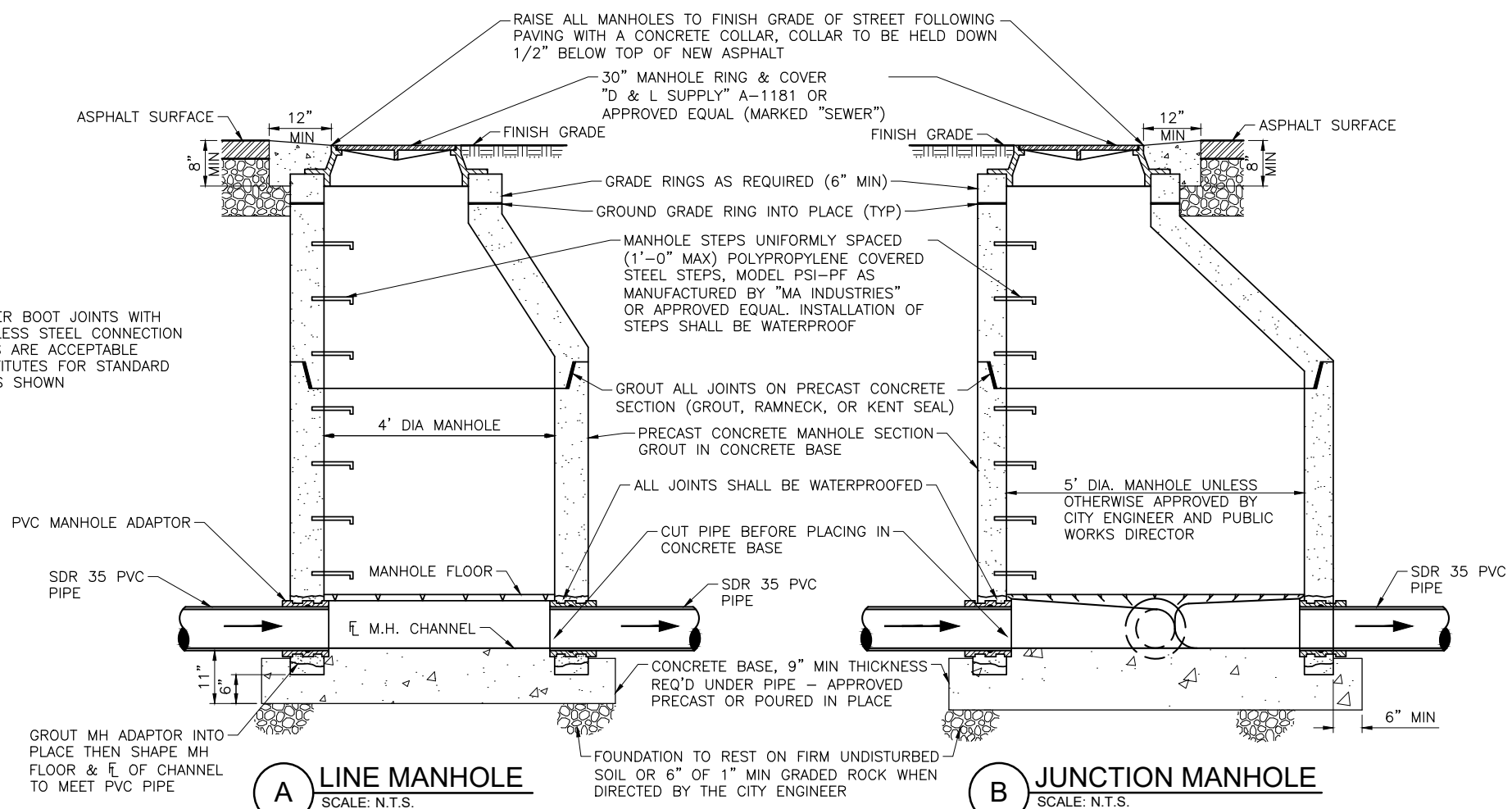


**LINE MANHOLE PLAN**

**JUNCTION MANHOLE PLAN**



**2 OUTSIDE DROP MANHOLE**  
TYPICAL D.I. PIPE TO CONCRETE PIPE CONNECTION  
SCALE: N.T.S.



**A LINE MANHOLE**  
SCALE: N.T.S.

**B JUNCTION MANHOLE**  
SCALE: N.T.S.

- NOTES:
1. ALL DROP MANHOLES REQUIRE SPECIAL APPROVAL FROM CITY ENGINEER OR PUBLIC WORKS DIRECTOR
  2. FOR CONSTRUCTION DIMENSION OF DROP MANHOLE, SEE JUNCTION MANHOLE OR LINE MANHOLE DETAIL ON THIS SHEET
  3. ALL PIPE FOR DROP MANHOLE TO BE DUCTILE IRON PIPE, OR ALTERNATE SDR 35 PVC ENCASED IN CONCRETE FITTINGS TO BE MECHANICAL JOINT WITH MEGA LUG RESTRAINTS

NOTE:  
RUBBER BOOT JOINTS WITH STAINLESS STEEL CONNECTION BANDS ARE ACCEPTABLE SUBSTITUTES FOR STANDARD JOINTS SHOWN

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SANITARY SEWER MANHOLE DETAILS

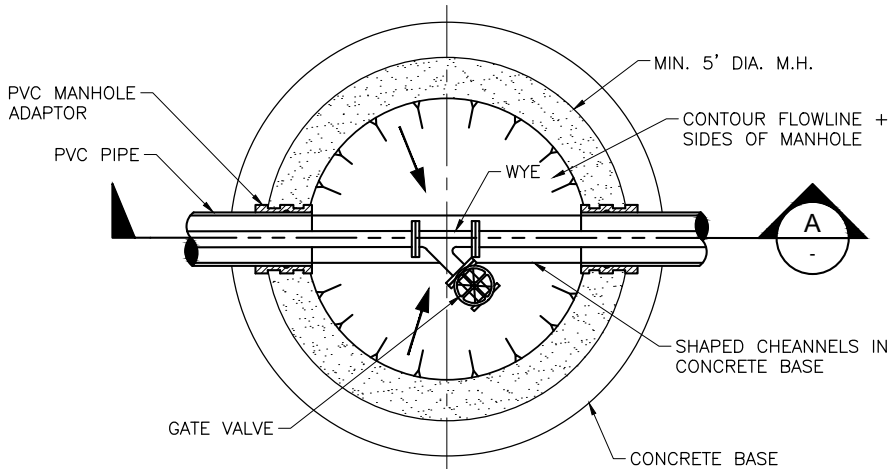
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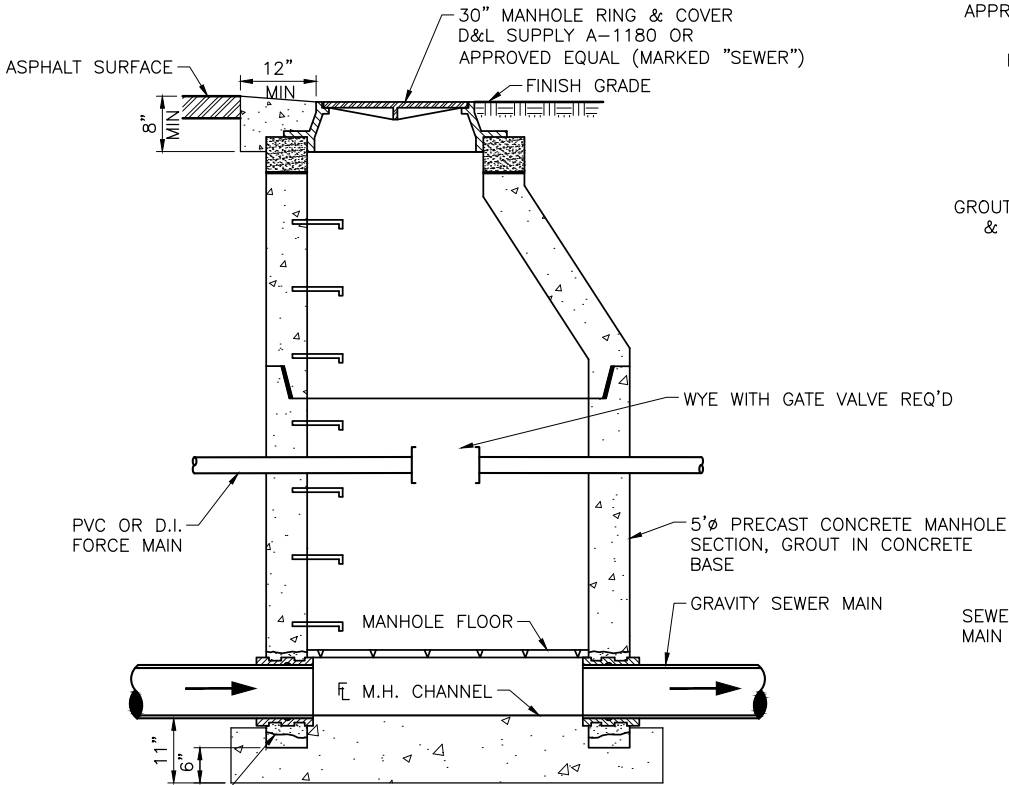
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PIPE & FITTING SCHEDULE		
NO.	DESCRIPTION	FITTING
A	2" WASTEWATER AIR RELEASE VALVE, VAL Matic MODEL #49A W/ OPTIONAL VACUUM CHECK ON THE OUTLET	THR.
B	2" BRASS PIPE	THR.
C	2" BALL VALVE (1/4 TURN 200 PSI MIN.	THR.
D	2" NYLON COATED SERVICE SADDLE W/ STEEL STRAPS ROMAC MODEL #202 NS	-

NOTE: WHERE SEWAGE WATER QUALITY IS ADEQUATE AS DICTATED BY THE CITY ENGINEER, AN A.R.I. D040 OR APPROVED EQUAL VALVE MAY BE SPECIFIED.

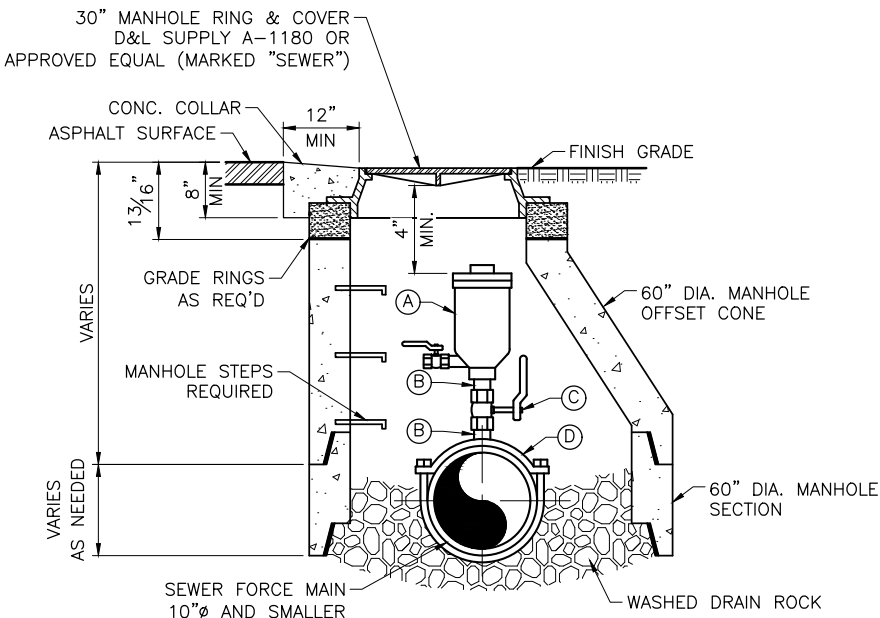


PLAN

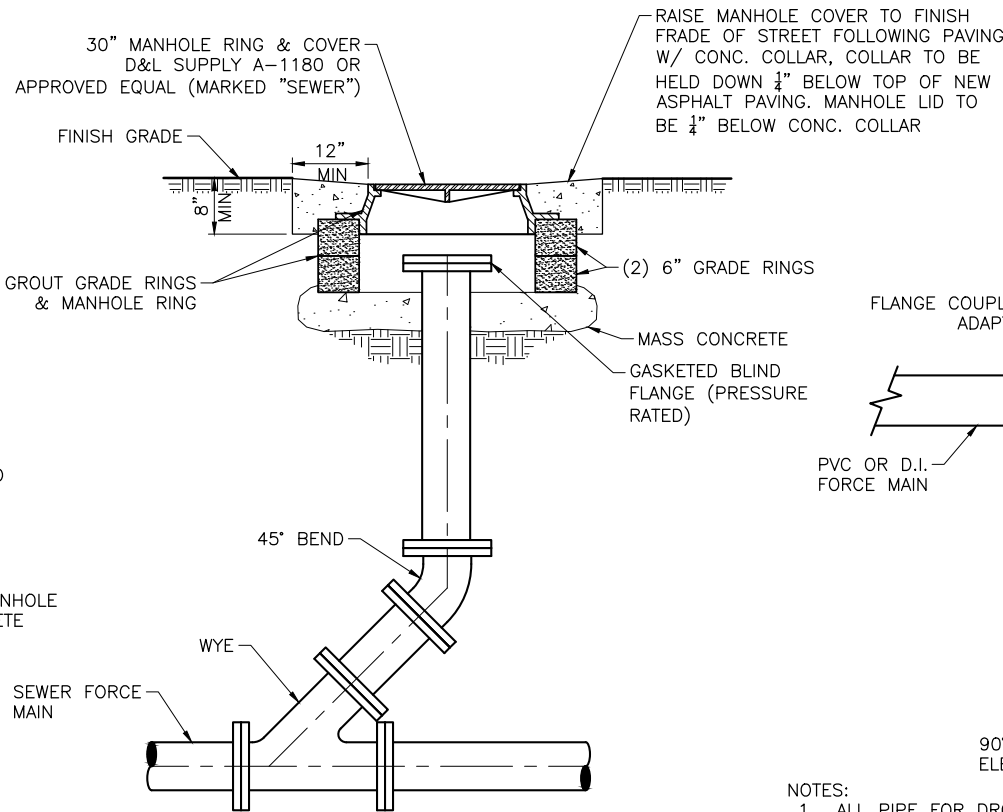


GROUT MH ADAPTOR INTO PLACE THEN SHAPE MH FLOOR &  $\nabla$  OF CHANNEL TO MEET PVC PIPE

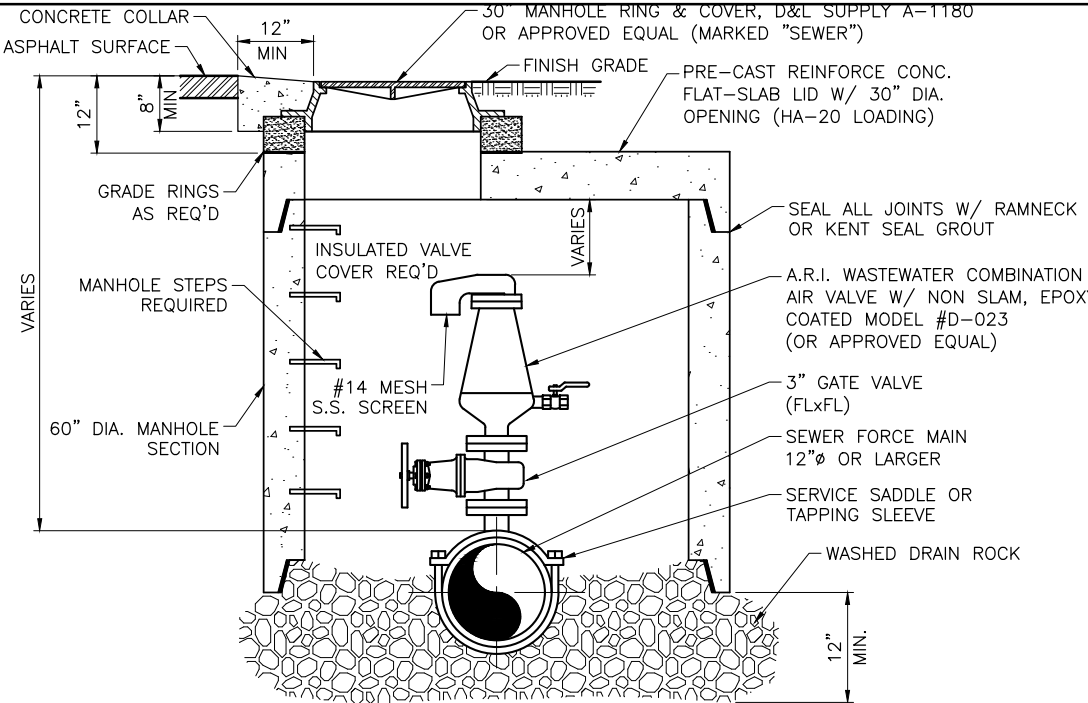
**A** **FORCE MAIN CLEANOUT MANHOLE**  
WHERE SPECIFIED BY THE WASTEWATER TREATMANGER



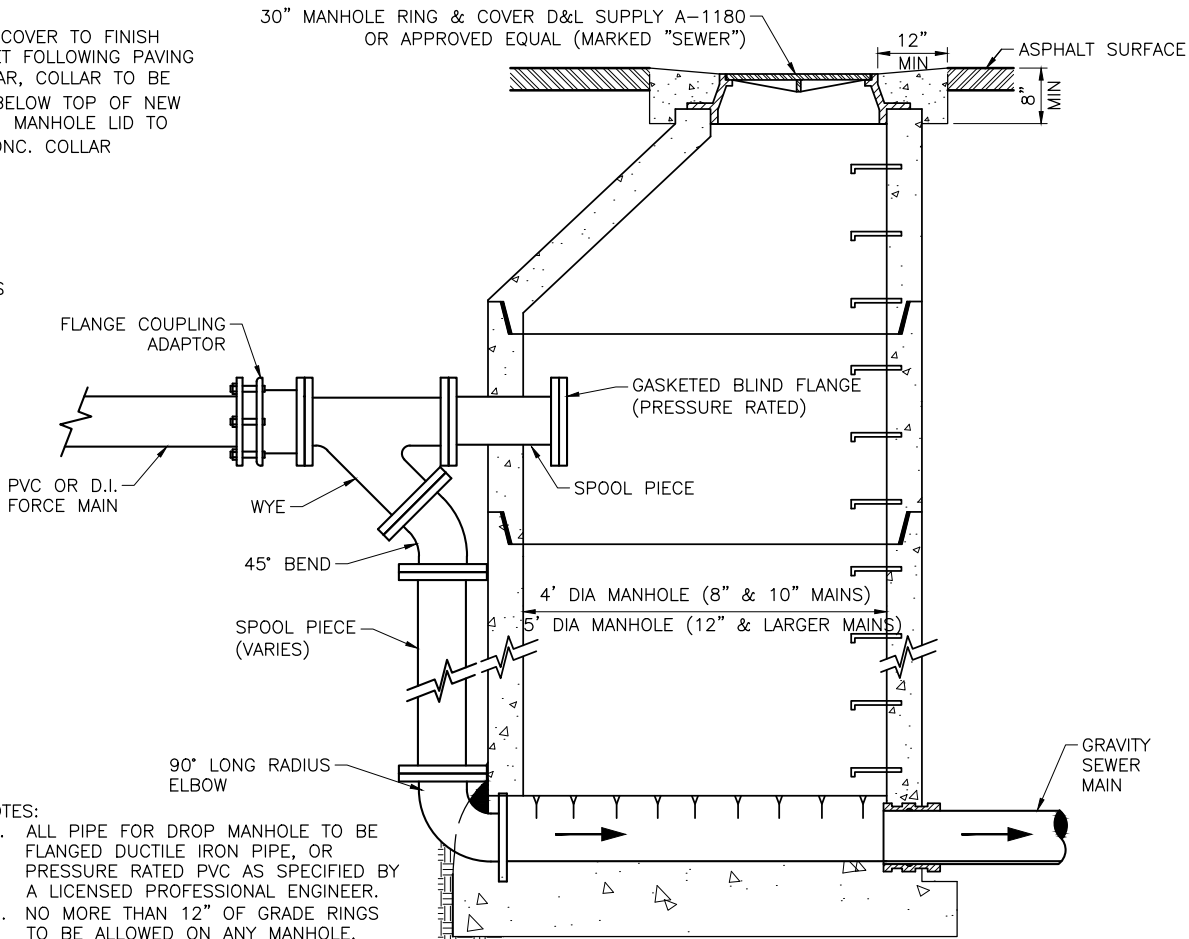
**2" WASTEWATER AIR RELEASE VALVE**  
10" Ø MAINS AND UNDER



**STANDARD FORCE MAIN CLEANOUT**  
4"Ø TO 12"Ø MAINS



**3" WASTEWATER AIR/VACUUM RELIEF VALVE**  
12"Ø MAINS AND LARGER



- NOTES:
- ALL PIPE FOR DROP MANHOLE TO BE FLANGED DUCTILE IRON PIPE, OR PRESSURE RATED PVC AS SPECIFIED BY A LICENSED PROFESSIONAL ENGINEER.
  - NO MORE THAN 12" OF GRADE RINGS TO BE ALLOWED ON ANY MANHOLE.

**FORCE MAIN CONNECTION MANHOLE**  
FORCE MAIN CONNECTION TO GRAVITY SEWER

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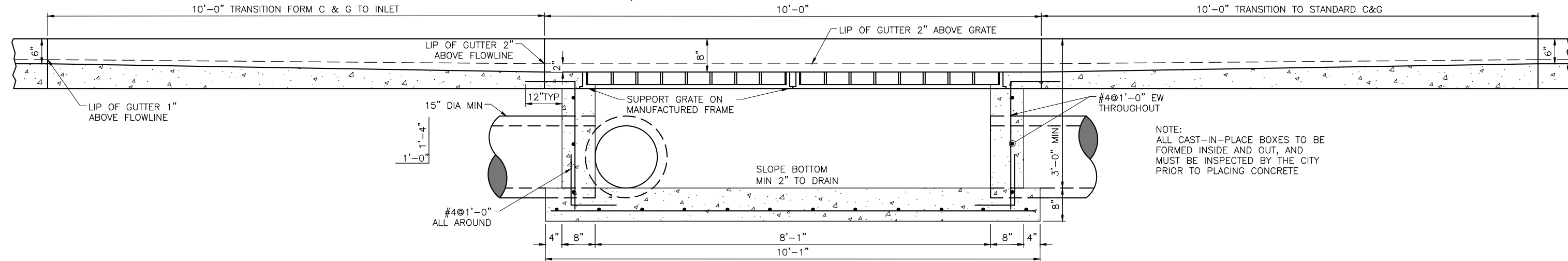
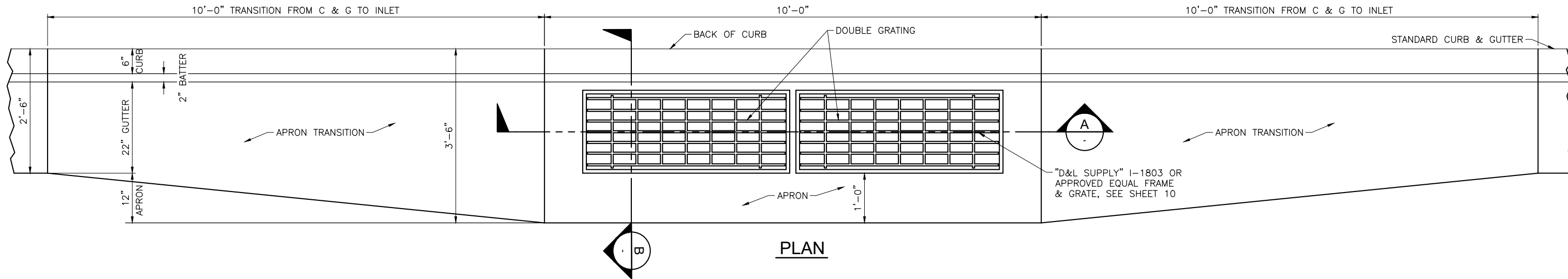
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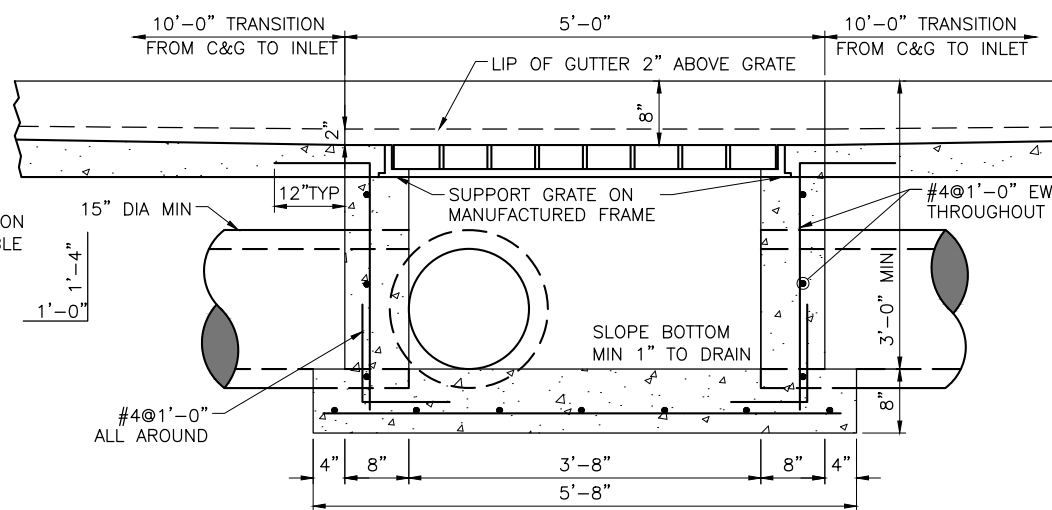
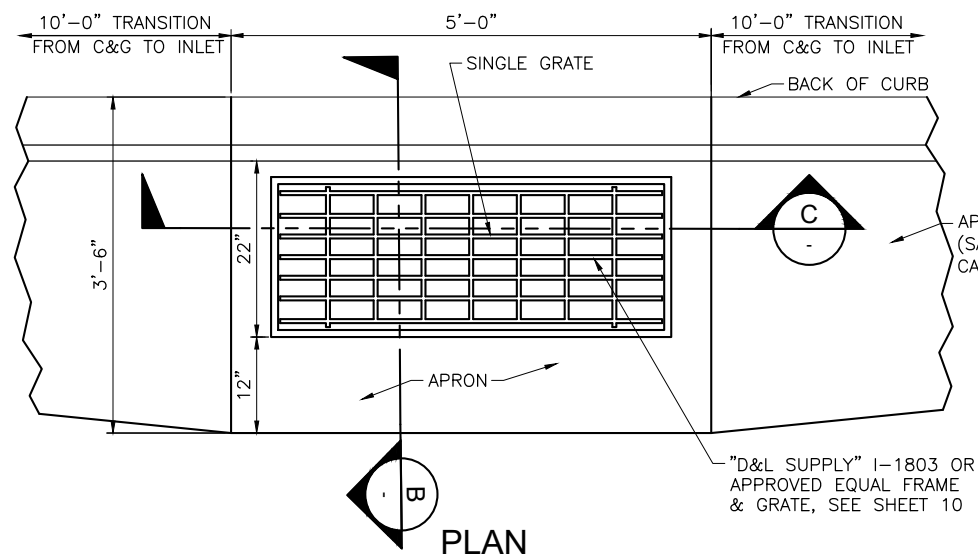
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SANITARY SEWER FORCE MAIN DETAILS	

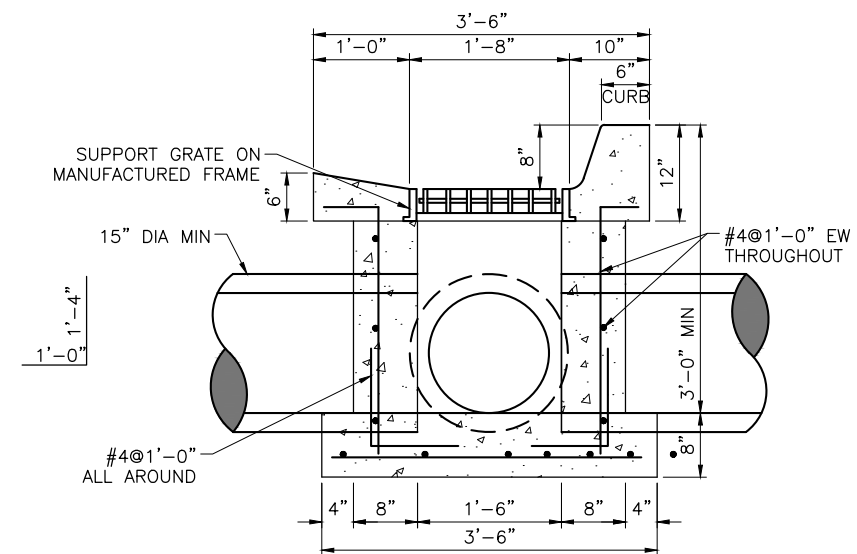
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**A** DOUBLE CATCH BASIN  
SCALE: N.T.S.



**C** SINGLE CATCH BASIN  
SCALE: N.T.S.



**B** SECTION B  
SCALE: N.T.S.

Plot Date: 4/16/2020 1:41 PM Plotted By: Daniel Johnson  
Drawing Created: 2/27/2020 JUB-8-101-C-PROJECT SUBMITTAL CITY STANDARDS 2020 CITY STANDARDS.DWG

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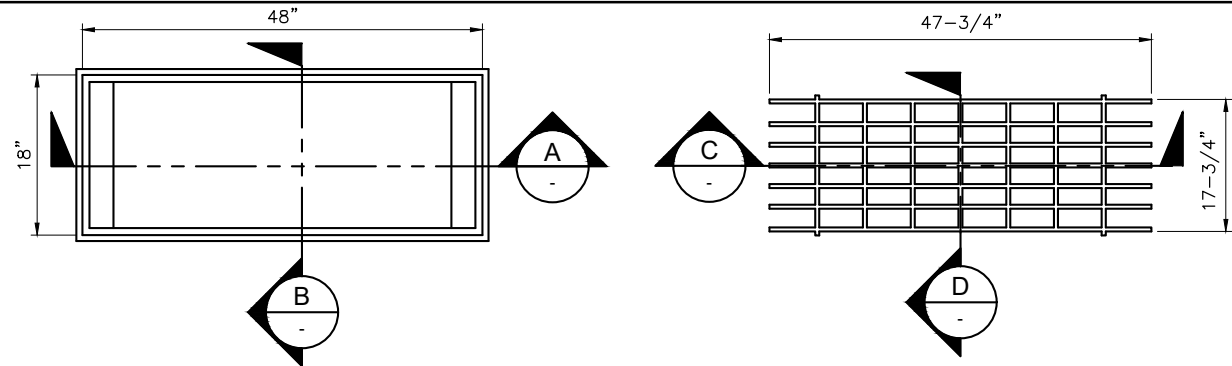
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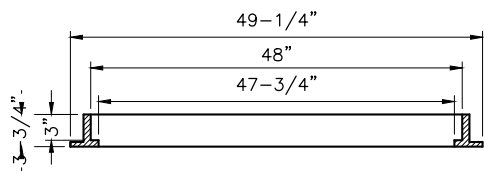
DOUBLE & SINGLE CATCH BASIN DETAILS

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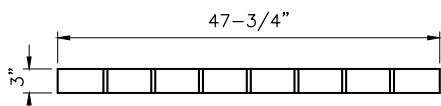


**1 FRAME DETAIL**  
SCALE: N.T.S.

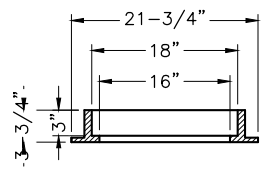
**2 GRATE DETAIL**  
SCALE: N.T.S.



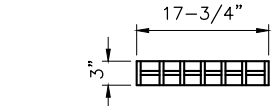
**A FRAME DETAIL**  
SCALE: N.T.S.



**C GRATE DETAIL**  
SCALE: N.T.S.

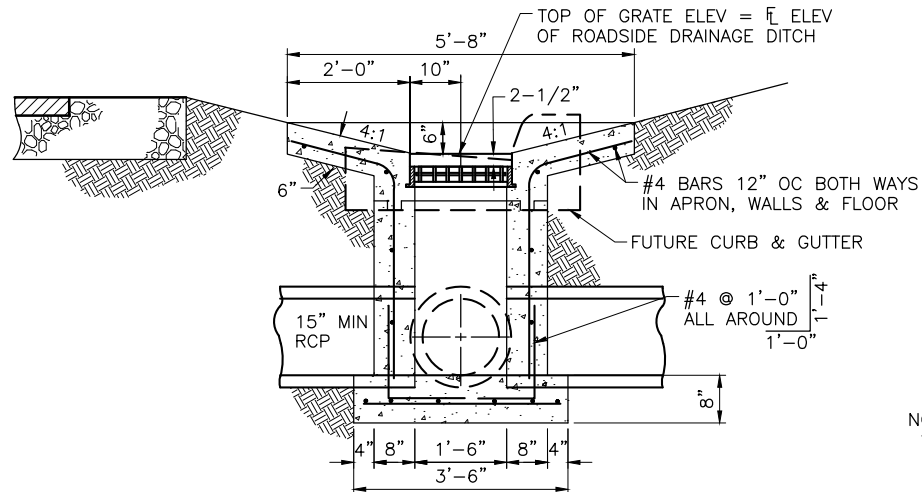


**B FRAME DETAIL**  
SCALE: N.T.S.



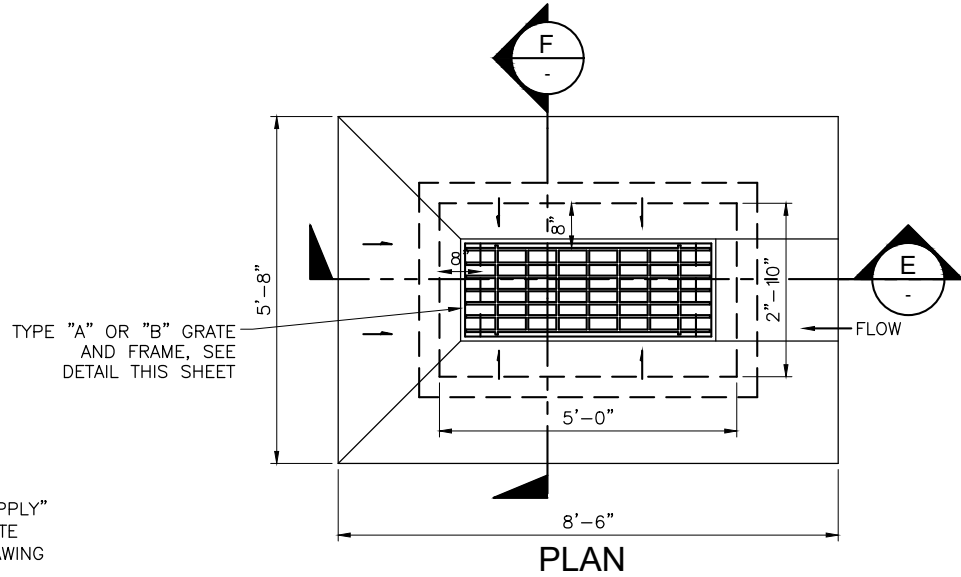
**D GRATE DETAIL**  
SCALE: N.T.S.

**3 STANDARD FRAME & GRATE**  
SCALE: N.T.S.



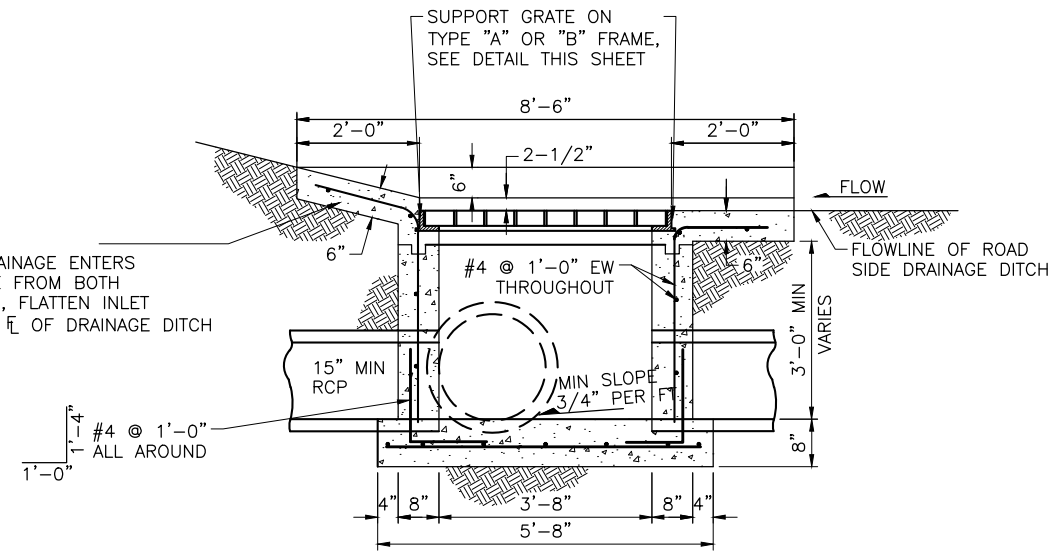
**F CURBLESS INLET**  
SCALE: N.T.S.

- NOTE:
1. TYPE "A" FRAME & GRATE SHALL BE "D&L SUPPLY" 1-1803 OR OPTIONAL TYPE "B" FRAME & GRATE SHALL BE UDOT BICYCLE-SAFE (STANDARD DRAWING #1703) AS APPROVED BY CITY ENGINEER
  2. BICYCLE-SAFE GRATE REQUIRED
  3. FRAME & GRATE SHALL BE PRE-FABRICATED AND GALVANIZED IN ACCORDANCE WITH ASTM A-123 SPECIFICATIONS



**PLAN**

- NOTE:
- WHERE DRAINAGE ENTERS STRUCTURE FROM BOTH DIRECTIONS, FLATTEN INLET APRON TO  $\bar{\ell}$  OF DRAINAGE DITCH



**E INLET**  
SCALE: N.T.S.

- NOTES:
1. ALL BOXES ARE TO BE FORMED INSIDE & OUT AND ARE TO BE INSPECTED BY THE CITY PRIOR TO PLACING CONCRETE
  2. CITY ENGINEER SHALL SPECIFY WHICH STANDARD GRATE (TYPE "A" OR "B") IS TO BE INSTALLED IN EACH INDIVIDUAL CATCH BASIN STRUCTURE

Plot Date: 4/16/2020 1:41 PM Plotted By: Daniel Johnson  
File Name: C:\Users\jeb\OneDrive\Documents\PROJECTS\PLAIN CITY STANDARDS\2020 CITY STANDARDS.DWG

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Kaysville, Utah 84037

Phone: 801.547.0393  
Fax: 801.547.0397  
www.jub.com



AGENCY

REVIEW	FILE: CITY STANDARDS
	JUB PROJ. # : ----
	DRAWN BY: DTJ
	DESIGN BY: PJT
	CHECKED BY: PJT
	ONE INCH AT FULL SIZE, IF NOT ONE INCH, SCALE ACCORDINGLY

PUBLIC WORKS STANDARDS  
PLAIN CITY CORPORATION

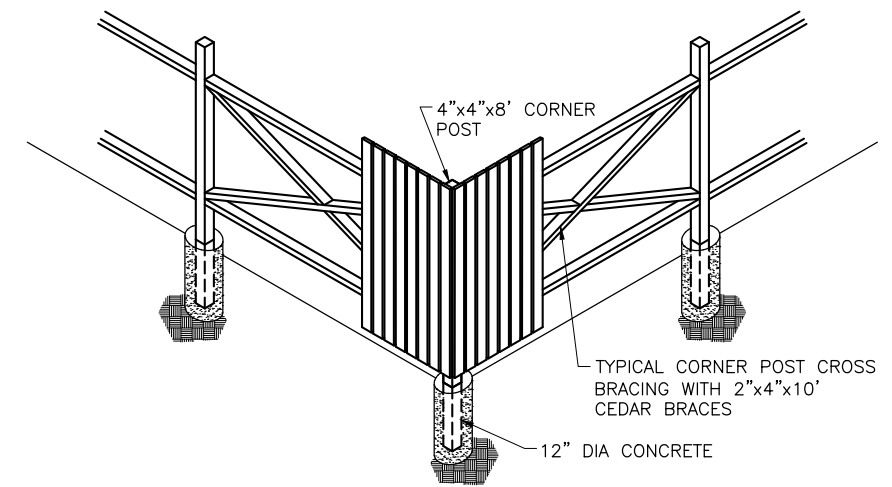
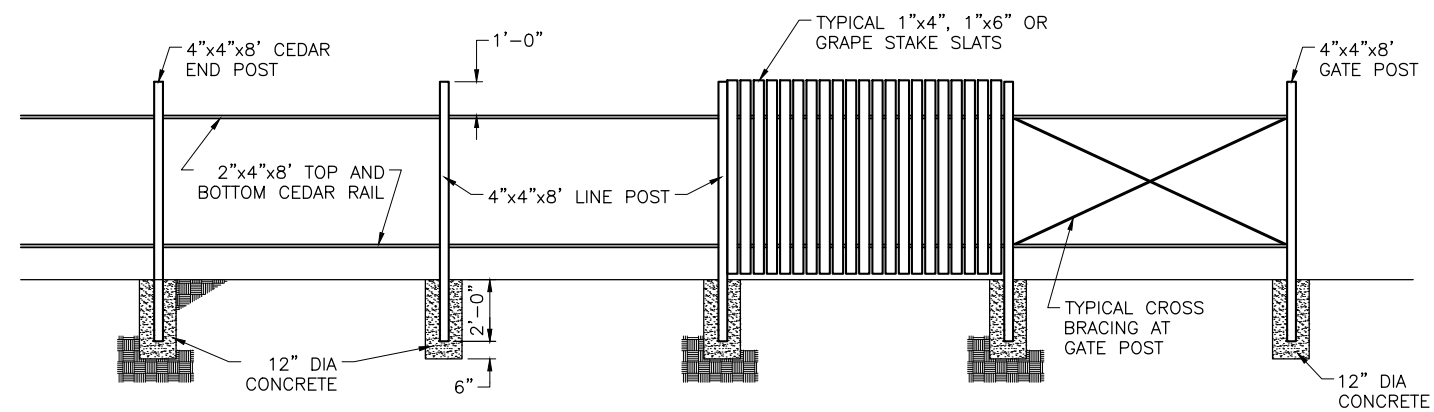
CURBLESS INLET AND FRAME & GRATE DETAILS

LAST UPDATED: 4/16/2020  
SHEET NUMBER:

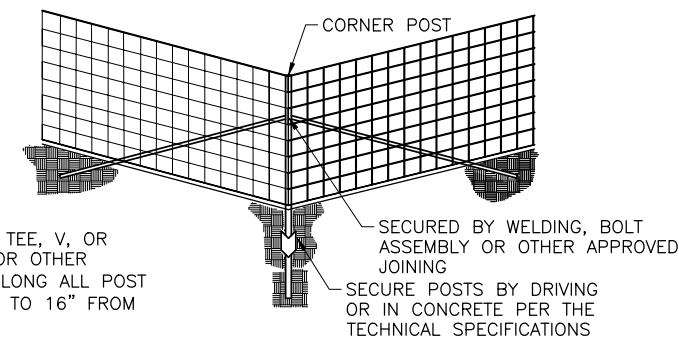
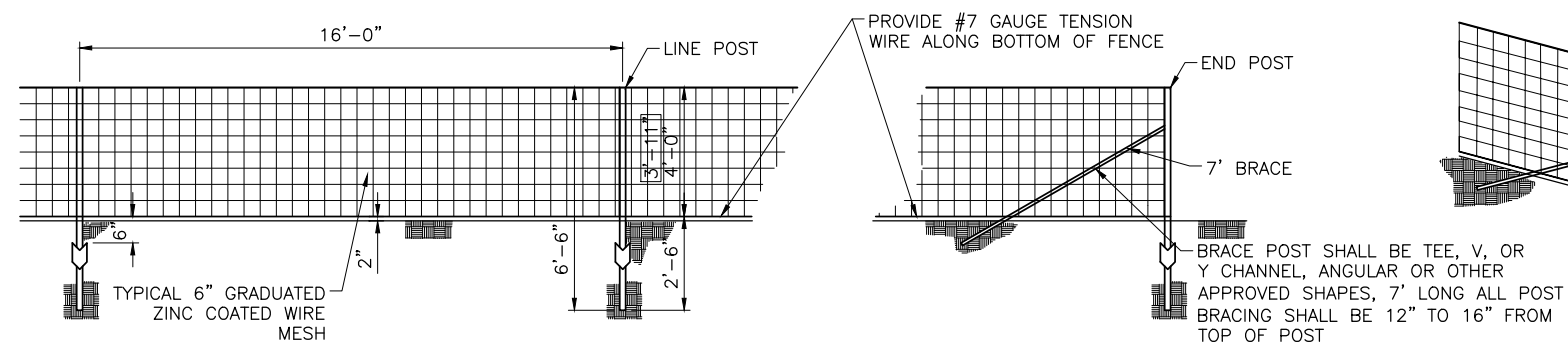
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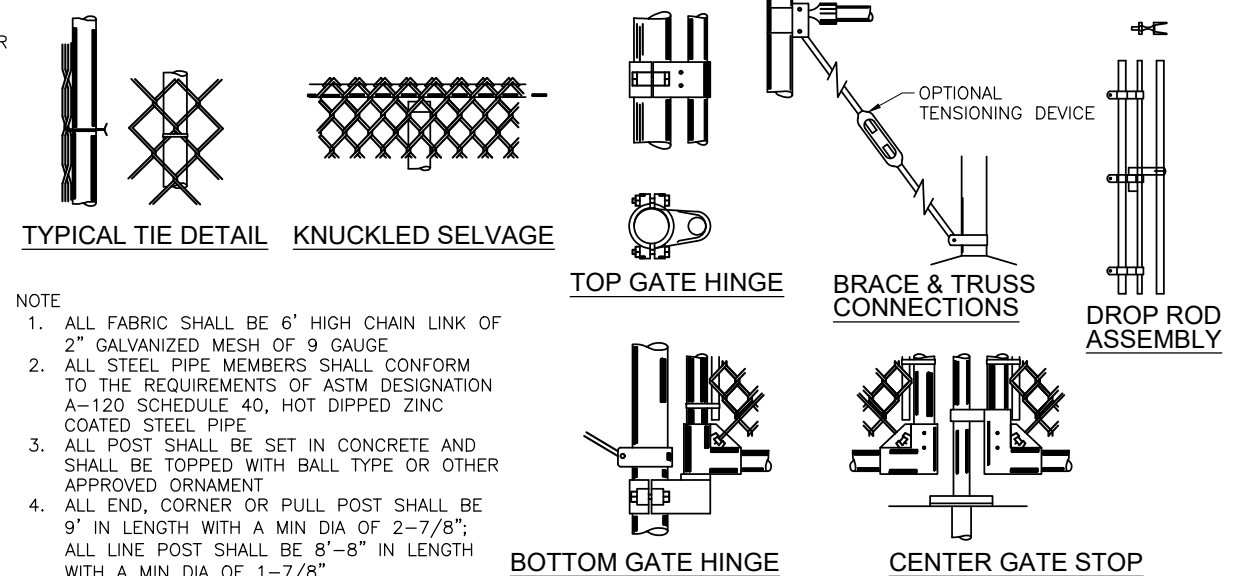
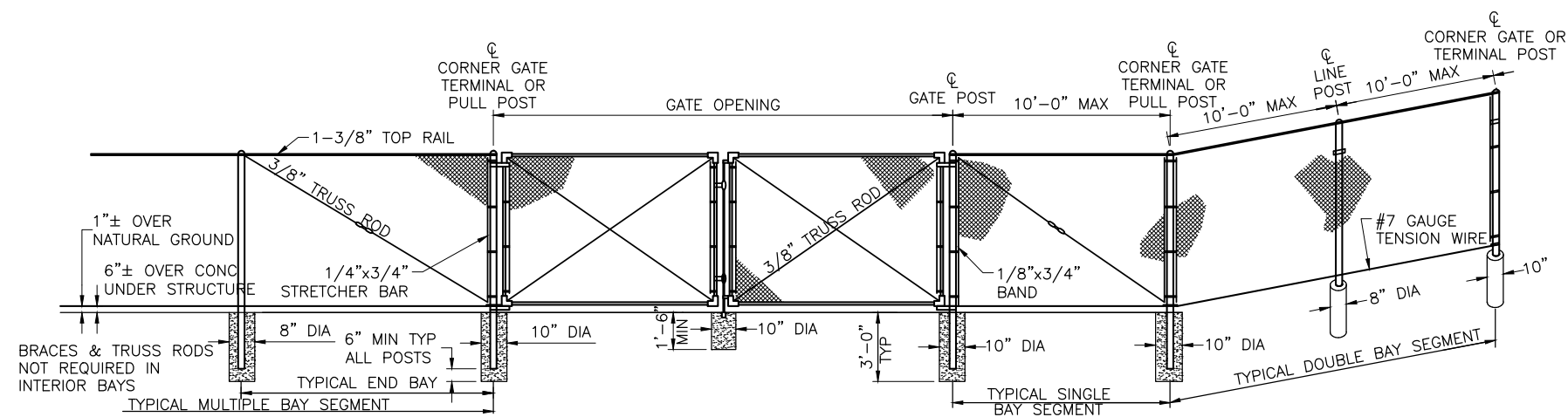




NOTE:  
THIS SHEET SPECIFIES VARIOUS FENCING TYPES  
TO BE CONSTRUCTED WHEN CALLED FOR BY THE  
CITY ENGINEER



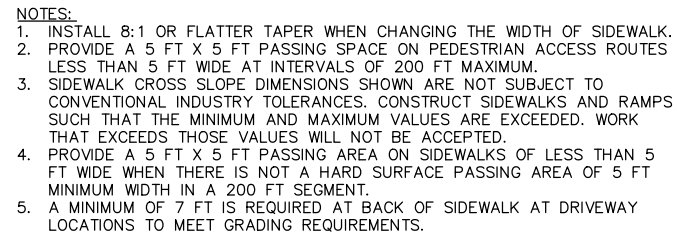
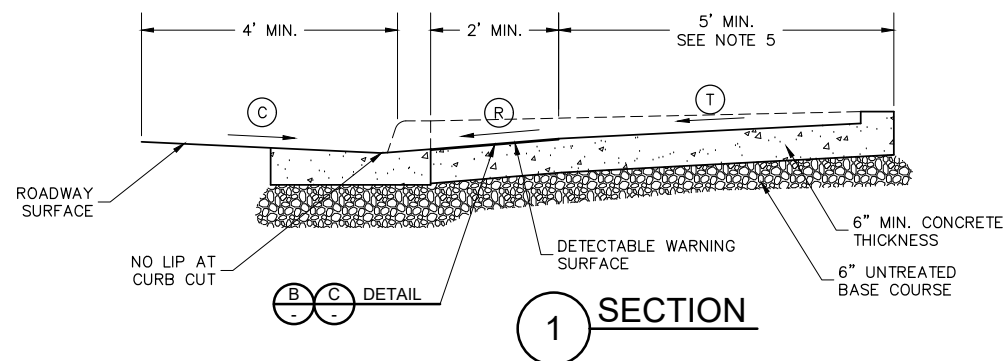
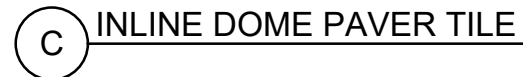
GATES			
GATE POST AND GATE FRAMES			
HEIGHT	GATE OPENING	GATE POST	GATE FRAME
6 FEET AND OVER	SINGLE TO 6' OR DOUBLE TO 12'	2-5/8"	1-1/2"
	SINGLE OVER 6' TO 13' OR DOUBLE OVER 12' TO 24'	3-1/2"	
	SINGLE OVER 13' TO 18' OR DOUBLE OVER 24' TO 36'	6"	
	SINGLE OVER 18' OR DOUBLE OVER 36'	8"	



NOTE

1. ALL FABRIC SHALL BE 6' HIGH CHAIN LINK OF 2" GALVANIZED MESH OF 9 GAUGE.
2. ALL STEEL PIPE MEMBERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM DESIGNATION A-120 SCHEDULE 40, HOT DIPPED ZINC COATED STEEL PIPE
3. ALL POST SHALL BE SET IN CONCRETE AND SHALL BE TOPPED WITH BALL TYPE OR OTHER APPROVED ORNAMENT
4. ALL END, CORNER OR PULL POST SHALL BE 9' IN LENGTH WITH A MIN DIA OF 2-7/8"; ALL LINE POST SHALL BE 8'-8" IN LENGTH WITH A MIN DIA OF 1-7/8"

NOTE:  
ALL FENCING SHALL CONFORM TO LOCATION  
AND HEIGHT LIMITATIONS AS STATED IN THE  
PLAIN CITY FENCING ORDINANCE

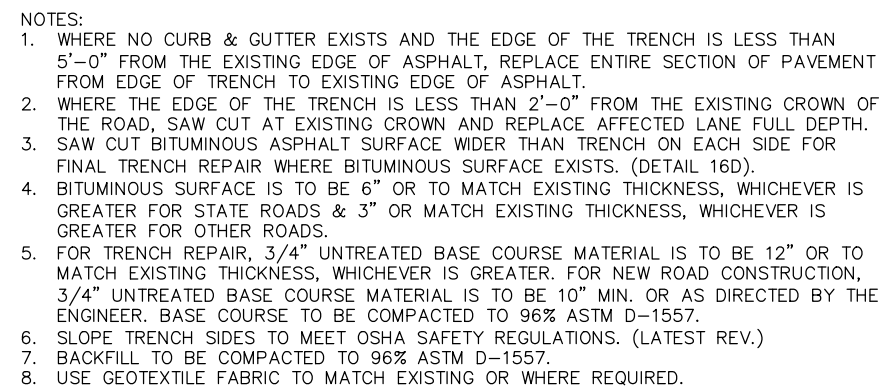


## D PARK STRIP DETAIL

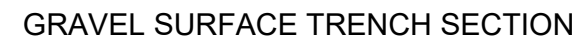
SLOPE TABLE			
	ITEM	MAX RUNNING SLOPE *	MAX CROSS SLOPE *
(T)	TURNING SPACE	2%	2% (d)
(R)	RAMP	8.3% (a) 5.1% MIN	2%
(B)	BLENDED TRANSITION	5%	2% (d)
(C)	CLEAR SPACE/GUTTER	5% (b)	2% (d)
(S)	SIDEWALK	—	2%
(F1)	FLARE WITHIN SIDEWALK	10% (c)	—
(F2)	FLARE NOT IN SIDEWALK	25% (C)	—
	CROSSWALK	5%	2% (e) (F)

(f) DO NOT EXCEED A CROSS SLOPE EQUAL TO THE STREET OR HIGHWAY GRADE AT MID BLOCK CROSSWALKS.

14. CLEAR SPACE SIZE: USE A 4 FT MINIMUM DEPTH AND THE LARGER OF THE CURB CUT WIDTH OR A 4 FT MINIMUM WIDTH.

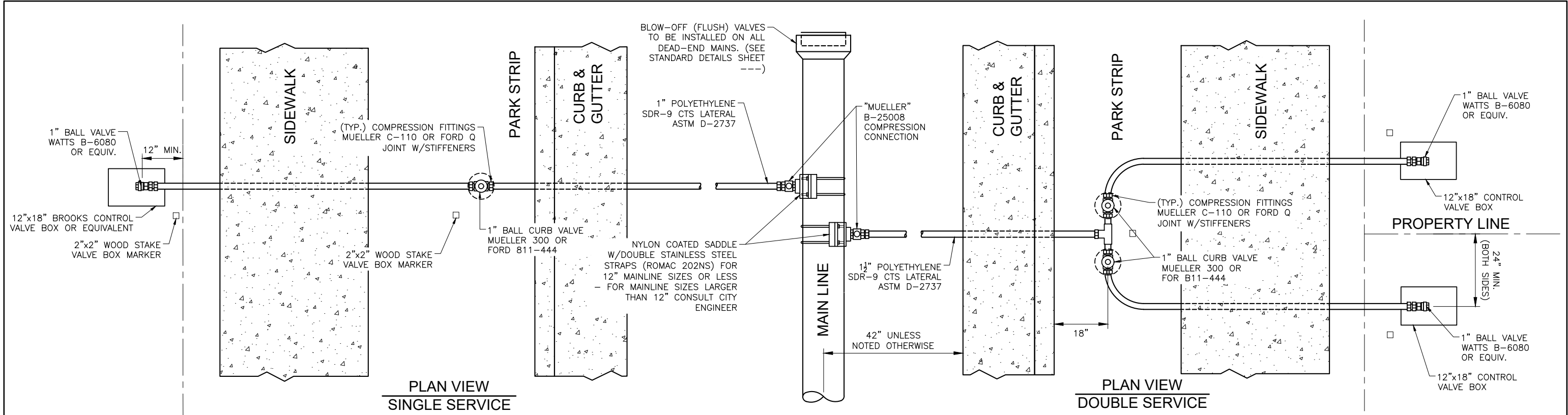


SCALE: N.T.S.



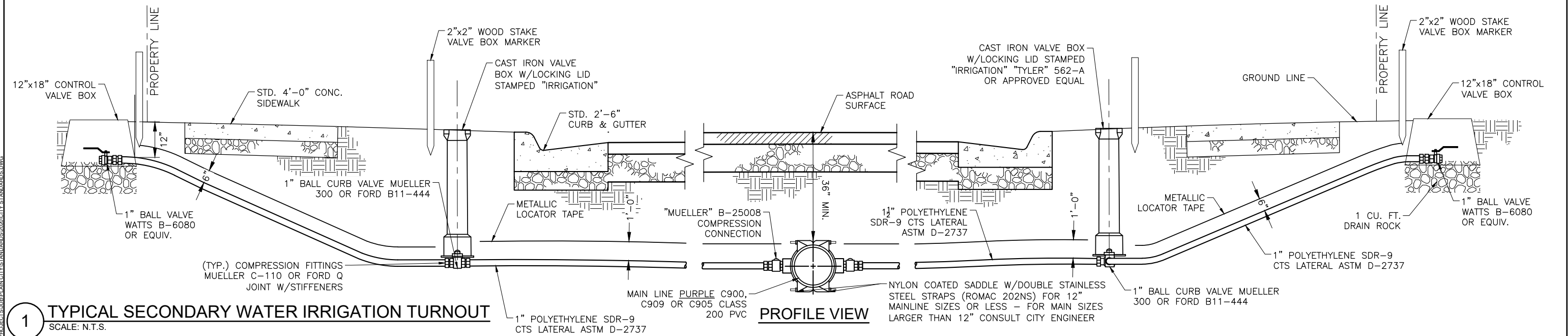
- NOTES:
1. SLOPE TRENCH SIDES TO MEET OSHA SAFETY REGULATIONS. (LATEST REV.)
  2. BACKFILL TO BE COMPACTED TO 96% ASTM D-1557 IN ROADWAYS AND 90% IN LANDSCAPED AREAS.

SCALE: N.T.S.



NOTES:

1. 1 1/2" & 1" PIPE SHALL BE POLYETHYLENE SDR-9 CTS, ASTM D-2737 W/PURPLE 14 GAUGE TRACER WIRE.
2. VALVE BOX MARKERS MUST BE SET ADJACENT TO EVERY VALVE.
3. SERVICE PIPE SHALL MAINTAIN A MINIMUM BURY DEPTH OF 36" UP TO CURB STOP; 12" MINIMUM AT CONTROL VALVE BOX.
4. FITTINGS SHALL BE STAINLESS UNLESS SPECIFIED OTHERWISE.
5. ALL VALVE LIDS SHALL BE STAMPED "IRRIGATION"
6. CONCRETE COLLARS SHALL BE SQUARE.
7. SERVICE LINES ARE TO BE TAPPED LEVEL.
8. ALL MAIN LINES ARE TO BE PURPLE C900 W/PURPLE 14 GAUGE TRACER WIRE & INSTALLED TO LIMITS OF SUBDIVISION (OR STREET) & PLUGGED UNTIL FUTURE CONNECTION TO CITY IRRIGATION SYSTEM.
9. PLUG ALL OPEN ENDS OF PIPE UNTIL FUTURE CONNECTION TO CITY SECONDARY WATER SYSTEM.
10. ALL SECONDARY WATER SYSTEM PRESSURES SHALL BE A MINIMUM OF 10 PSI BELOW THE CULINARY WATER SYSTEM PRESSURES AT COMMON LOCATIONS.
11. ALL SPECIFIED BRANDS OF MATERIALS SHOWN ON THESE DRAWINGS ARE "CITY STANDARDS." OTHER EQUIVALENT BRANDS MAY BE USED WITH THE PRIOR APPROVAL OF THE CITY ENGINEER AND THE CITY WATER DIVISION SUPERVISOR.



1 TYPICAL SECONDARY WATER IRRIGATION TURNOUT  
SCALE: N.T.S.

Plot Date: 4/16/2020 1:42 PM Plotted By: Daniel Johnson  
File Name: 27-2020 - JUB-811-444 - PROJECT SUBMITTAL CITY STANDARDS 2020 CITY STANDARDS.DWG

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REVISION			
NO.	DESCRIPTION	BY	DATE

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J-U-B ENGINEERS, INC.

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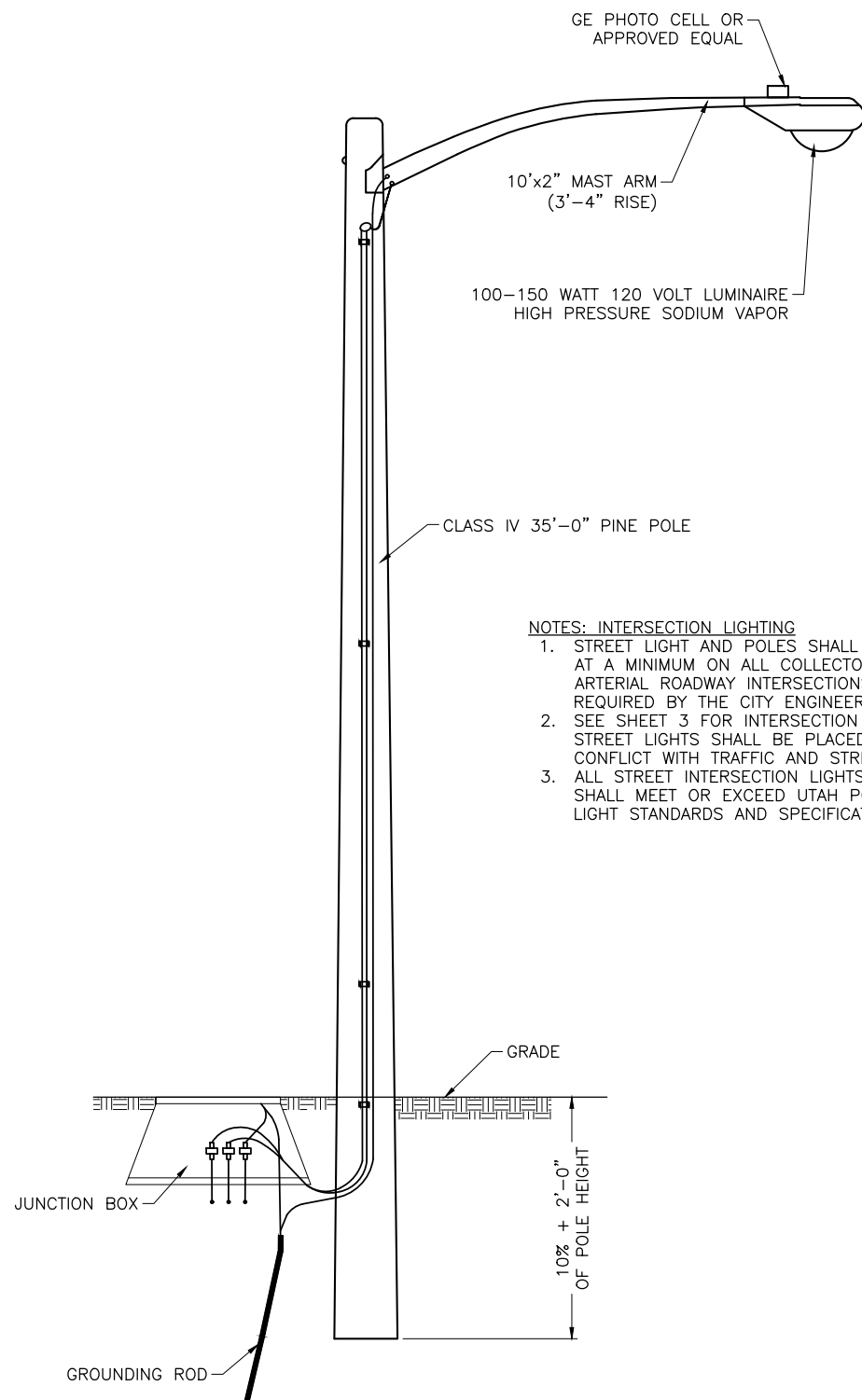
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	JUB PROJ. # : ---
	DRAWN BY: DTJ
	DESIGN BY: PJT
	CHECKED BY: PJT
ONE INCH AT FULL SIZE, IF NOT ONE INCH, SCALE ACCORDINGLY	

PUBLIC WORKS STANDARDS  
PLAIN CITY CORPORATION

TYPICAL SECONDARY WATER IRRIGATION TURNOUT

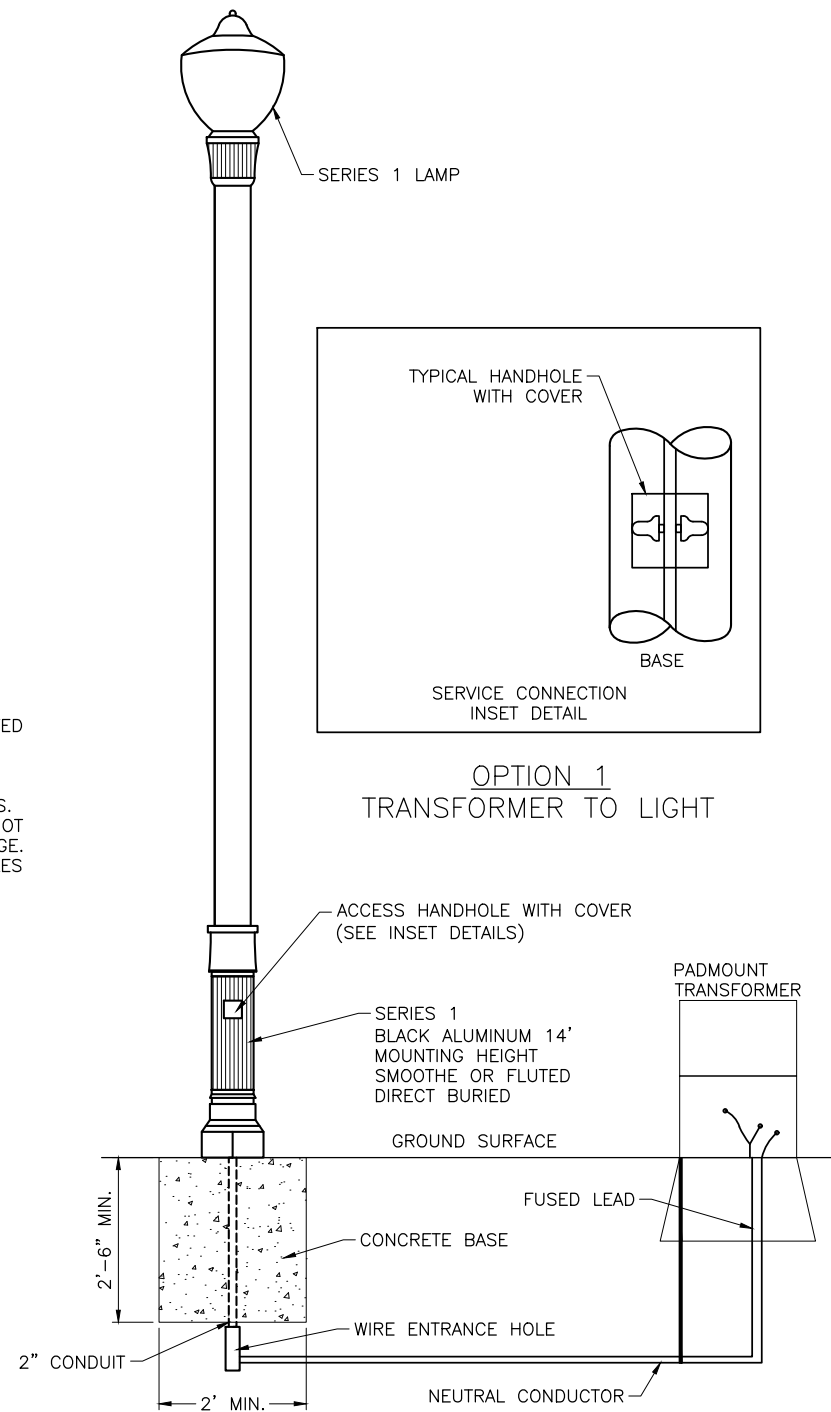
LAST UPDATED: 4/16/2020  
SHEET NUMBER:  
17

Plot Date: 4/16/2020 1:42 PM Plotted By: Daniel Johnson  
File Name: 2/2/2020 JUB-8 PUBLIC WORKS STANDARDS PLAIN CITY STANDARDS.DWG



**1 STREET INTERSECTION LIGHT POLE DETAIL**  
SCALE: N.T.S.

- NOTES: INTERSECTION LIGHTING**
1. STREET LIGHT AND POLES SHALL BE LOCATED AT A MINIMUM ON ALL COLLECTOR AND ARTERIAL ROADWAY INTERSECTIONS OR AS REQUIRED BY THE CITY ENGINEER
  2. SEE SHEET 3 FOR INTERSECTION LOCATIONS. STREET LIGHTS SHALL BE PLACED AS TO NOT CONFLICT WITH TRAFFIC AND STREET SIGNAGE.
  3. ALL STREET INTERSECTION LIGHTS AND POLES SHALL MEET OR EXCEED UTAH POWER AND LIGHT STANDARDS AND SPECIFICATIONS.



**2 STREET LIGHTING LUMINAIRE AND POLE ASSEMBLY DETAIL**  
SCALE: N.T.S.

**PROFILE VIEW**

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FILE: CITY STANDARDS
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REVIEW
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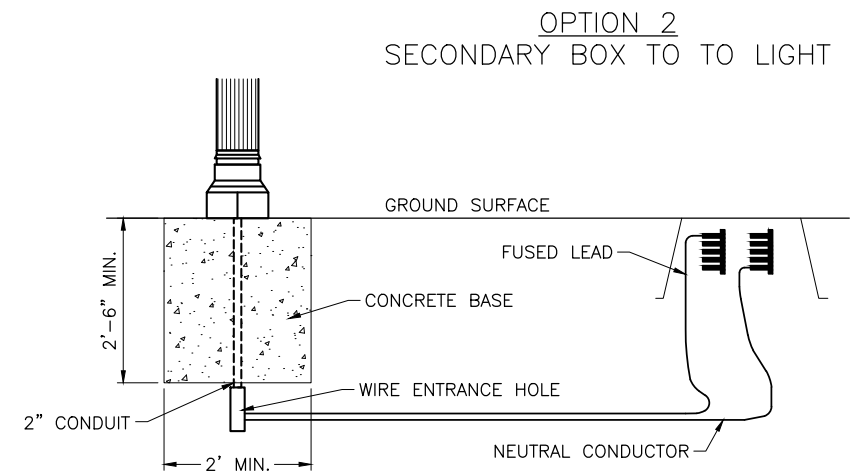
PUBLIC WORKS STANDARDS  
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STREET LIGHTING DETAILS

LAST UPDATED: 4/16/2020

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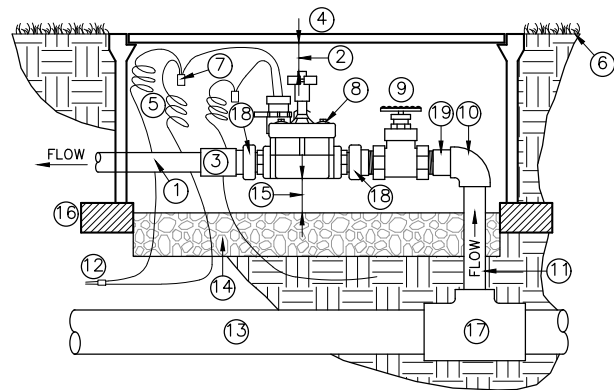
18



**3 STREET LIGHTING BELOW GRADE ELECTRICAL CONNECTION**  
SCALE: N.T.S.

- NOTES: STREET LIGHTING**
1. STREET LIGHT AND POLES SHALL BE LOCATED AS REQUIRED AND/OR APPROVED BY THE CITY ENGINEER. STREET LIGHTING POLE LOCATIONS SHALL NOT CONFLICT WITH TRAFFIC AND STREET SIGNAGE
  2. ALL STREET LIGHTS AND POLES SHALL MEET OR EXCEED UTAH POWER AND LIGHT STANDARDS AND SPECIFICATIONS.
  3. STREET LIGHTING SHALL BE DIRECTED DOWN TOWARDS THE RIGHT OF WAY AND NOT AT ONCOMING TRAFFIC.
  4. STREET LIGHTING WATTAGE SHALL BE LOW ENOUGH AS TO NOT CAUSE GLARING OR OBSTRUCT VEHICLE DRIVER VISION.

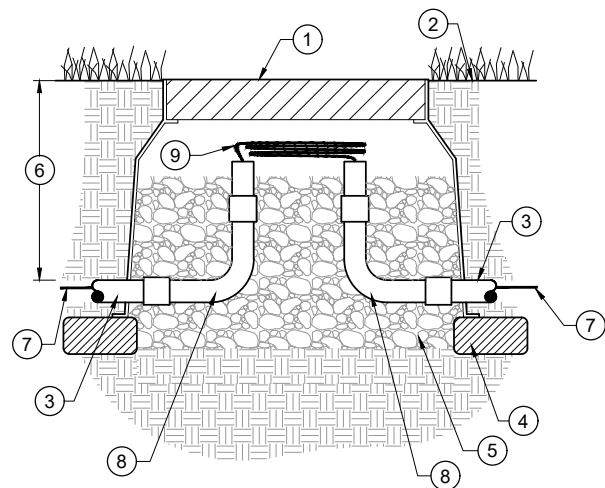




NOTES:  
1. ONLY ONE VALVE PER BOX.  
2. PLACE BOXES A MINIMUM OF 2' APART.  
3. USE BOX EXTENSIONS AS REQUIRED.

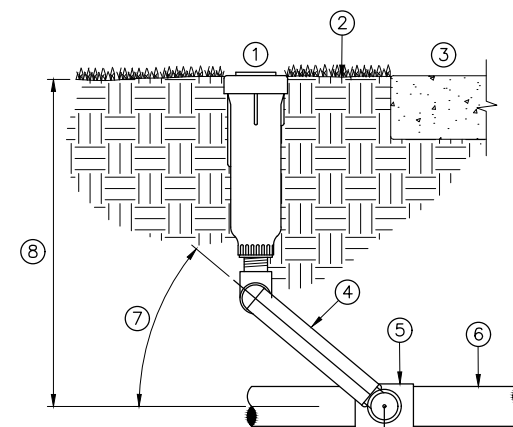
- ① PVC LATERAL LINE
- ② 3" MIN. 6" MAX. CLEARANCE
- ③ COUPLER SCH. 80 SLIP TO SLIP
- ④ PLASTIC VALVE BOX W/ BOLT LOCK (CARSON OR APPROVED EQUAL) SIZE AS PER VALVE SIZE: 1" VALVE = 18" STANDARD 1419 BOX; 1½"-2" VALVE = 18" JUMBO 1220 BOX
- ⑤ PROVIDE 24" EXPANSION LOOP AT EACH WIRE CONNECTOR IN BOX
- ⑥ FINISH GRADE
- ⑦ WATER TIGHT WIRE CONNECTORS (TYP)
- ⑧ VALVE - RAIN BIRD PESB VALVE
- ⑨ BRASS GATE VALVE (MUELLER, MATCO, NIBCO) WITH NON-RISING STEM
- ⑩ PVC SCH. 80 ELL
- ⑪ PVC SCH. 80 PIPE, SOLVENT WELDED, LENGTH AS REQUIRED
- ⑫ WIRES TO CONTROLLER, TAPE AND BUNDLE EVERY 10' - SEE TRENCH DETAIL
- ⑬ MAIN LINE
- ⑭ 4" MIN. DEPTH ¾" WASHED GRAVEL
- ⑮ 4" MIN. CLEARANCE REQUIRED
- ⑯ CONCRETE PAVERS ONLY
- ⑰ PVC SCH. 80 TEE (OR ELL OR DUCTILE IRON SERVICE TEE)
- ⑱ ACTION UNION - PART 18010-XX  
PART 18011-XX  
PART 18012-XX
- ⑲ PVC TOE NIPPLE

### CONTROL VALVE ASSEMBLY



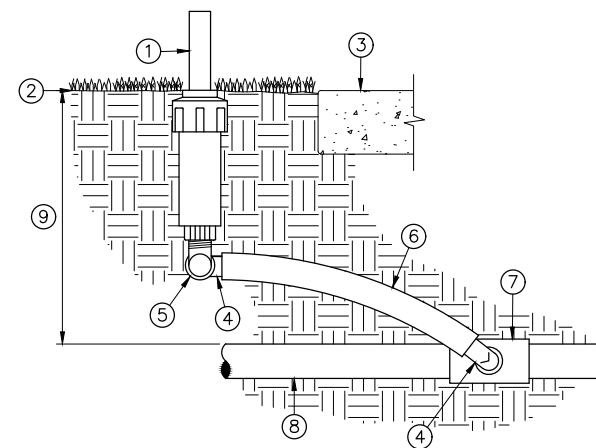
- ① 18" GREEN JUMBO 1220 VALVE BOX W/ BOLT LOCK (CARSON OR APPROVED EQUAL). USE BOX EXTENSIONS AS NEEDED.
- ② FINISHED GRADE
- ③ SCH. 40 PVC SLEEVE/CONDUIT (SEE IRRIGATION PLANS FOR SIZE)
- ④ CONCRETE PAVERS ONLY
- ⑤ PEA GRAVEL
- ⑥ DEPTH - 18" MIN.
- ⑦ 14 GAUGE IRRIGATION CONTROL WIRE
- ⑧ 90 DEGREE SWEEP ELL
- ⑨ 36" LOOP

### IRRIGATION CONTROL WIRE PULL BOX



- ① POP-UP ROTOR SPRINKLER LEGEND
- ② TOP OF SPRINKLER WILL BE FLUSH WITH GRADE
- ③ NOTE: ALL SPRAY HEADS TO BE PLACED 2" CLEAR OF ALL HARDSCAPE SURFACES
- ④ LASCO UNITIZED SWING JOINT OR SPEARS SWING JOINT RISER ASSEMBLY; 12" MIN. LENGTH; SIZE AS REQUIRED
- ⑤ PVC SCH 40 SxSxT TEE OR (ELL)
- ⑥ PVC LATERAL LINE, SIZE AS NOTED ON PLAN
- ⑦ SWING JOINT ARM INSTALLED AT ANGLE BETWEEN 30 AND 45 DEG. OF LATERAL PIPE
- ⑧ DEPTH - SEE NOTES & TRENCH DETAIL

### POP-UP GEAR DRIVE ROTOR SPRINKLER



- ① POP-UP SPRAY HEAD
- ② TOP OF SPRINKLER WILL BE FLUSH WITH GRADE
- ③ NOTE: ALL SPRAY HEADS TO BE PLACED 2" CLEAR OF ALL HARDSCAPE SURFACES
- ④ SWING PIPE ELL WITH SPIRAL BARB FITTING (TYP.)
- ⑤ MARLEX STREET ELL
- ⑥ FLEXIBLE SWING PIPE, 12" MIN., 36" MAX. LENGTH
- ⑦ PVC SCH 40 SxSxT TEE (OR ELL)
- ⑧ PVC LATERAL LINE, SIZE AS NOTED ON PLAN
- ⑨ DEPTH - SEE TRENCH DETAIL & SPECIFICATIONS

### POP-UP SPRAY/ROTARY SPRINKLER

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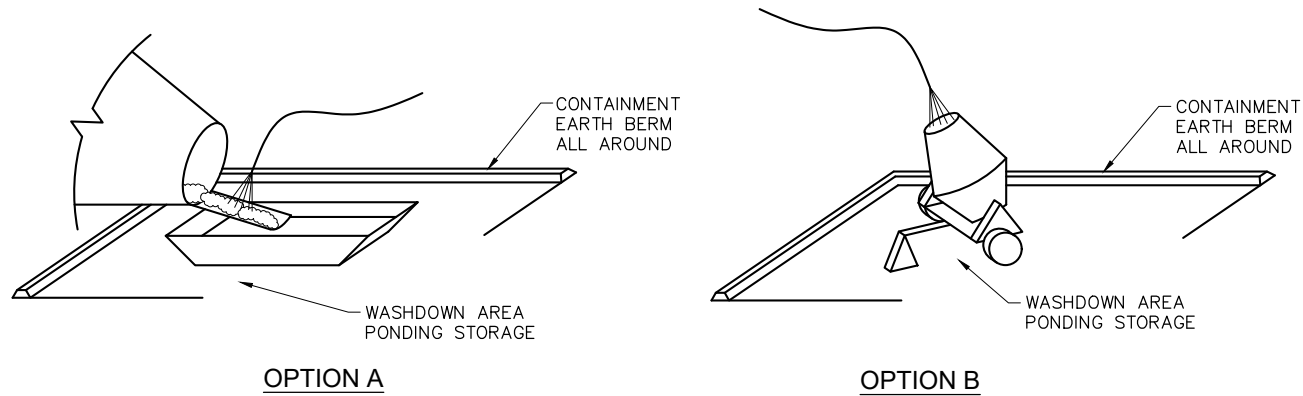
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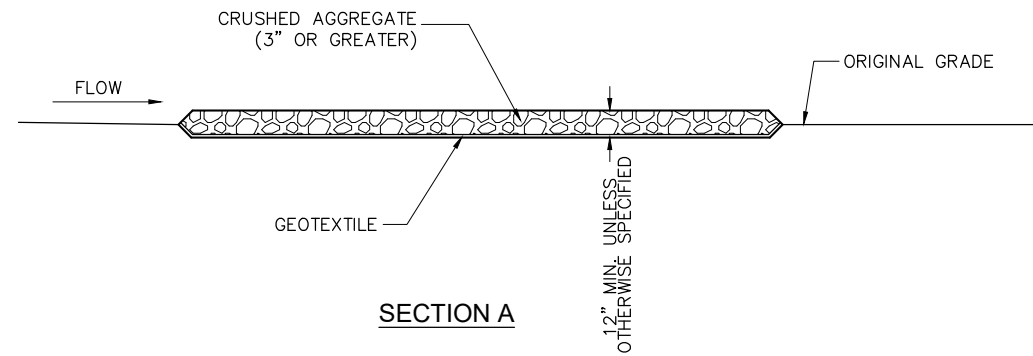
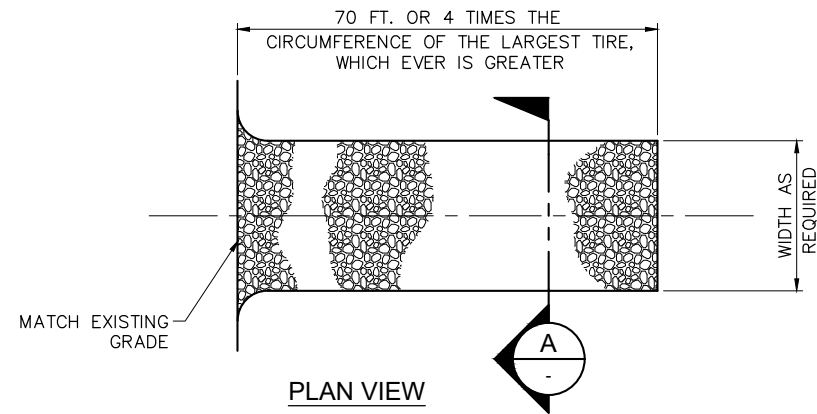
MUNICIPAL SPRINKLER SYSTEM DETAILS

LAST UPDATED: 4/16/2020  
SHEET NUMBER:  
**20**

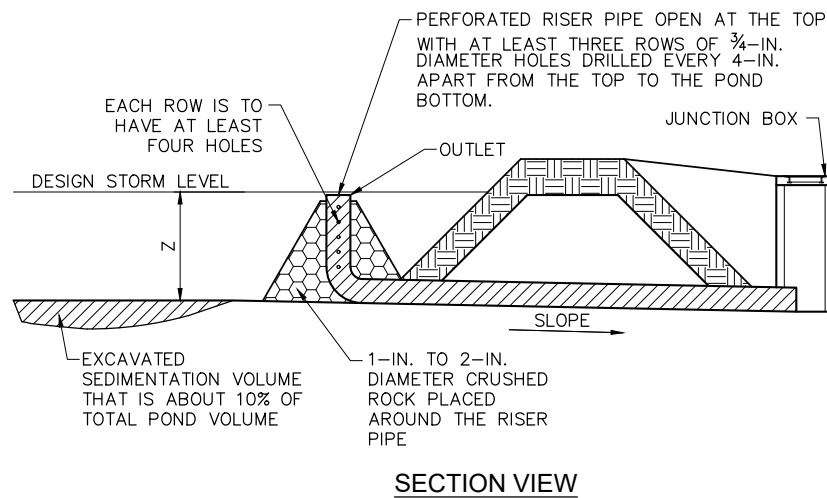
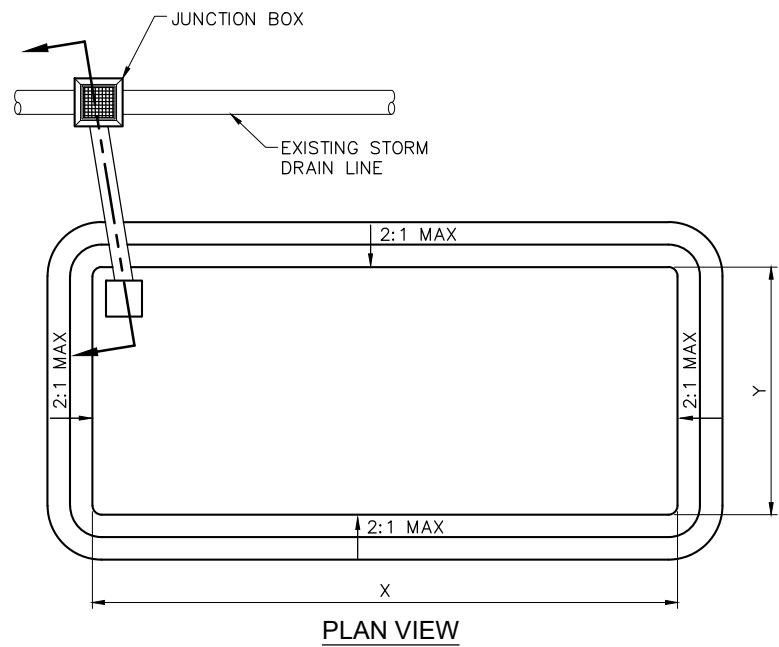




### CONCRETE WASTE MANAGEMENT



### CONTROLLED ACCESS DETAIL



SEDIMENT BASIN					
No.	X (min.)	Y (min.)	Z (depth min.)	OUTLET TYPE	DRAINAGE AREA
1	60'	20'	3'	RISER PIPE	6.6 ACRES±

### SEDIMENT BASIN DETAIL

Plot Date: 4/16/2020 1:42 PM Plotted By: Daniel Johnson  
 File: C:\Users\jeb\OneDrive\Documents\PROJECTS\PLAIN CITY STANDARDS\PROJECTS\PLAIN CITY STANDARDS.DWG

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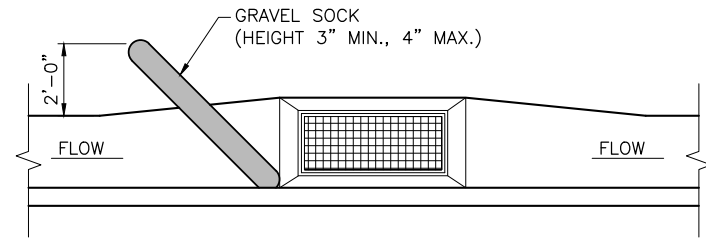
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 PLAIN CITY CORPORATION

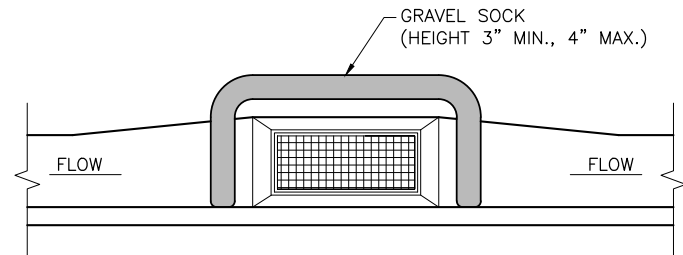
SWPPP DETAILS

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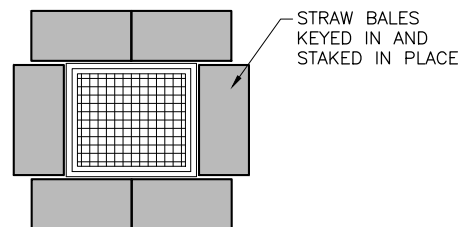
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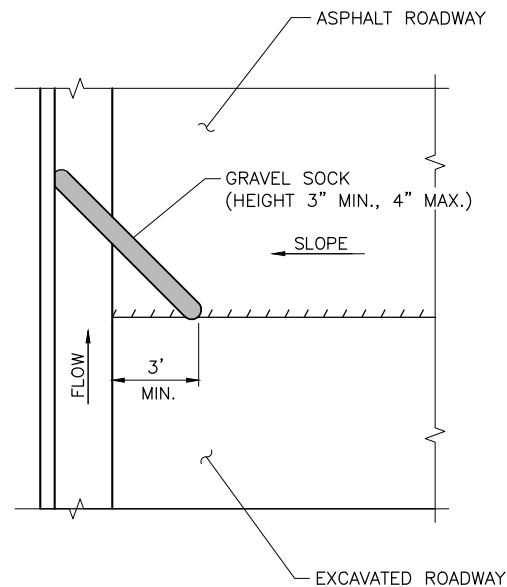
**ON-GRADE INLET PROTECTION DETAIL**



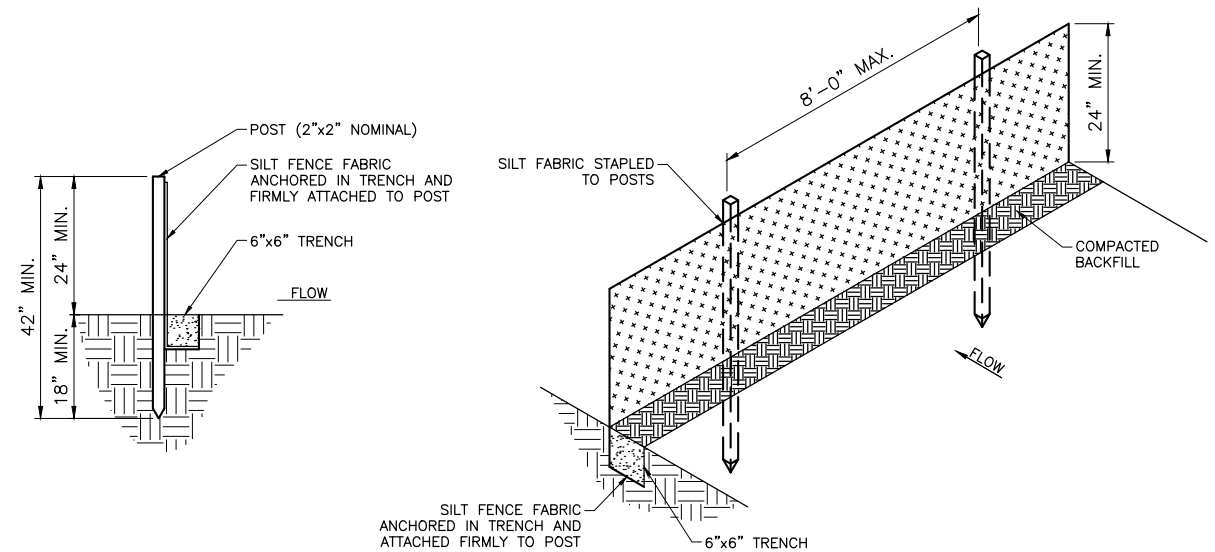
**LOW POINT INLET PROTECTION DETAIL**



**CENTER LOW POINT INLET PROTECTION DETAIL**

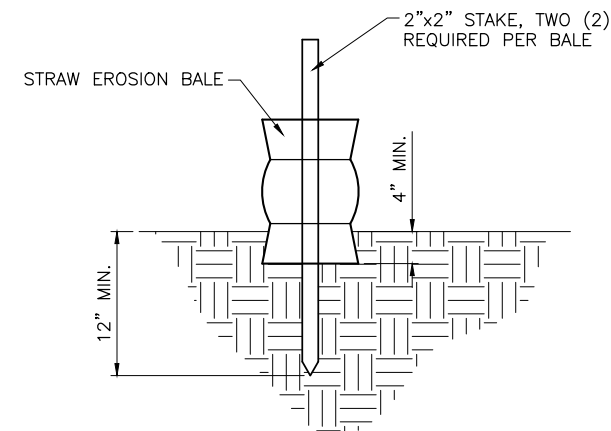


**EXCAVATED ROADWAY TO ASPHALT ROADWAY TRANSITION DETAIL**



- NOTES:
1. MINIMUM FILTER FABRIC HEIGHT SHALL BE 24".
  2. POSTS FOR SILT FENCES SHALL BE METAL OR HARD WOOD WITH A MINIMUM LENGTH OF 36". WOOD POSTS SHALL HAVE A MINIMUM DIAMETER OR CROSS SECTION OF 2". METAL POSTS SHALL BE "STUDDED TEE" OR "U" TYPE WITH MINIMUM WEIGHT OF 1.33 LBS/FOOT.
  3. DRIVE POSTS VERTICALLY INTO THE GROUND TO A MINIMUM DEPTH OF 18", AND EXCAVATE A TRENCH APPROXIMATELY 6" WIDE AND 6" DEEP ALONG THE LINE OF POSTS AND UPSLOPE FROM THE BARRIER. NO LESS THAN THE BOTTOM 1 FOOT OF THE FABRIC SHALL BE BURIED INTO THIS TRENCH.
  4. THE FILTER FABRIC MATERIALS SHALL BE FASTENED SECURELY TO METAL OR WOOD POSTS USING WIRE TIES, OR TO THE WOOD POSTS WITH ¾" LONG #9 HEAVY DUTY STAPLES.
  5. POSTS SHALL BE SPACED A MAXIMUM OF 8 FEET APART.

**SILT FENCE DETAIL**



**STRAW BAIL DETAIL**

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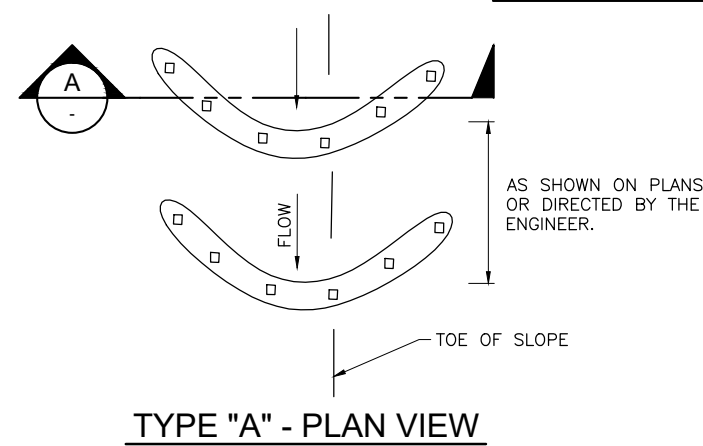
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	JUB PROJ. # :----
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	CHECKED BY: PJT
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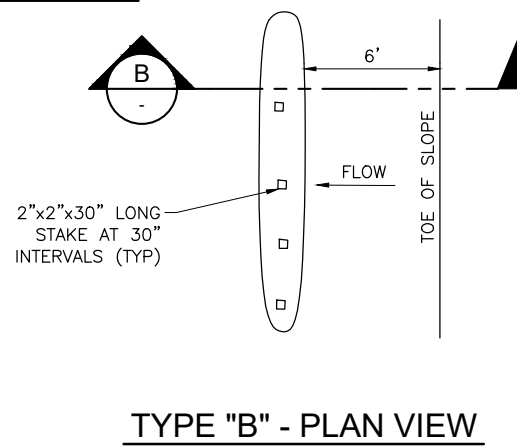
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<b>SWPPP DETAILS</b>	

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<b>22</b>

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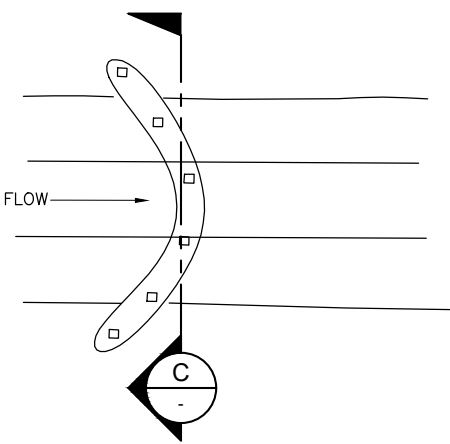


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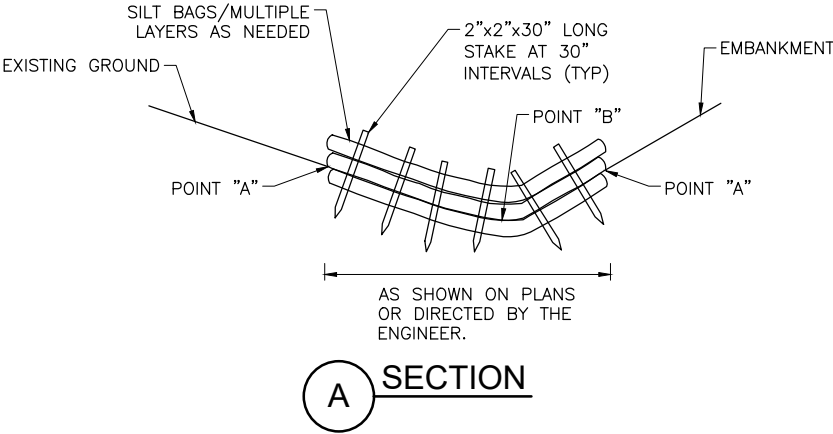


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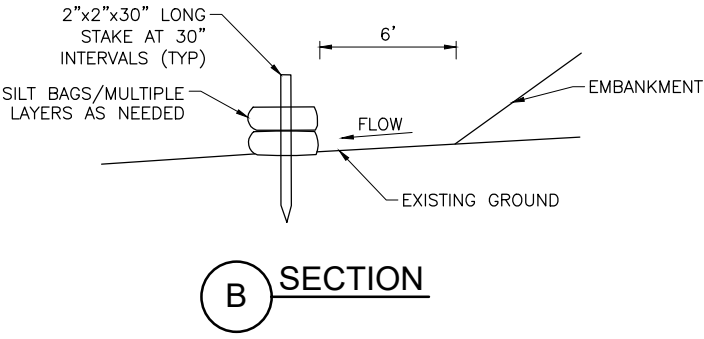
CHANNEL INSTALLATION



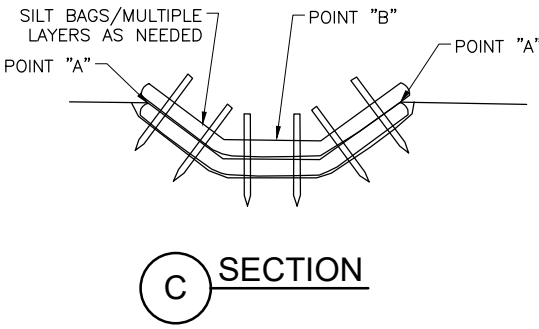
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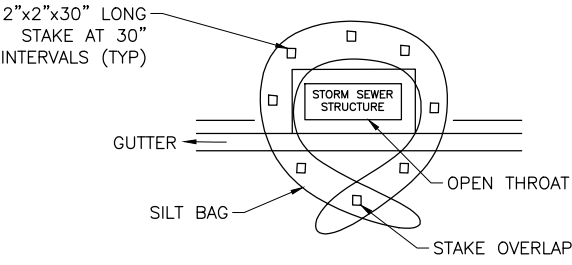
A SECTION



B SECTION

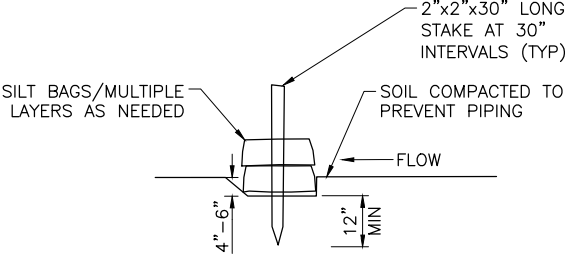


C SECTION



TEMPORARY BARRIER

- SILT BAG CHECK DAMS:**
1. DEFINITION: SILT BAGS USED AS MEANS OF CONTROLLING POLLUTION AND EROSION.
  2. PURPOSE: TO OBSTRUCT THE FLOW OF WATER TO ALLOW DEPOSIT OF SEDIMENT AND/OR DIVERT WATER TO A SLOPE DRAIN, SEDIMENT BASIN, SEDIMENT TRAP, OR OTHER EROSION CONTROL STRUCTURE.
  3. CONDITIONS WHERE APPLICABLE:
    - A. USE AT THE BOTTOM OF EMBANKMENT SLOPES TO DIVERT RUNOFF FROM SHEET FLOW AND ALSO CATCH SOME OF THE SEDIMENT PICKED UP IN THE SHEET FLOW.
    - B. AS CHECK DAMS IN SMALL DITCHES AND DRAINAGE AREAS.
    - C. ON THE LOWER SIDE OF CLEARED AREAS TO CATCH SEDIMENT FROM SHEET FLOW.
    - D. AS WATERPROOF CORES FOR STONE SEDIMENT DAMS.
  4. CONSTRUCTION METHODS: SILT BAGS ARE UTILIZED TO CONTROL EROSION, TRAP SEDIMENT, AND DIVERT RUNOFF. SILT BAGS MUST BE BRACED FROM BEHIND WHEN CONDITIONS REQUIRE.
  5. POINTS "A" MUST BE HIGHER THAN POINT "B".
  6. SILT BAG CHECK DAMS MAY BE USED IN SERIES WHEN SITE CONDITIONS REQUIRE OR AS DIRECTED BY CITY ENGINEER.



TYPICAL BALE INSTALLATION

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REUSE OF DRAWINGS			
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REVIEW	FILE: CITY STANDARDS
	JUB PROJ. # :----
	DRAWN BY: DTJ
	DESIGN BY: PJT
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SWPPP DETAILS	

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23

SANITARY SEWER:

- 1. PIPELINE INSPECTION – ALIGNMENT, GRADE, BEDDING & CLASS OF PIPE.
- 2. MANHOLE INSPECTION – CONTOUR FLOWLINE, BEDDING, STEPS, LIDS MARKED AND VENTED.
- 3. LOW PRESSURE AIR TEST OR INFILTRATION TEST.
- 4. DISPLACEMENT TEST & VIDEO RECORDING SHOWING DISTANCE, VIDEO SEWER FOR INITIAL ACCEPTANCE AND TO FIX ANY ISSUES THAT NEED TO BE FIXED. (LEADER WITH TIME, DATE & CONSTRUCTION COMPANY).
- 5. TO RELEASE SUBDIVISION FROM WARRANTY PHASE, SEWER MAIN MUST BE RE–VIDEOED AND APPROVED BY CITY REPRESENTATIVE.
- 6. THE PIPELINE MUST HAVE CLEAN WATER FLUSHED DOWN THE PIPES PRIOR TO VIDEOTAPING IN ORDER TO DETECT AND IDENTIFY ALL LOW SPOTS AND/OR BELLIES. FAILURE TO RUN WATER PRIOR TO VIDEOTAPING WILL RESULT IN REJECTION OF THE VIDEO TAPE TESTING.

STORM DRAINAGE

- 1. PIPELINE INSPECTION – ALIGNMENT, GRADE & CLASS OF PIPE, BEDDING.
- 2. CATCH BASIN BOXES INSPECTION – DEPTH, PLACEMENT AND REBAR.
- 3. PIPE INSPECTION – VIDEO RECORDING SHOWING DISTANCE, VIDEO STORM DRAIN FOR INITIAL ACCEPTANCE AND TO FIX ANY ISSUES THAT NEED TO BE FIXED (LEADER WITH TIME, DATE & CONSTRUCTION COMPANY).
- 4. THE PIPELINE MUST HAVE CLEAN WATER FLUSHED DOWN THE PIPES PRIOR TO VIDEOTAPING IN ORDER TO DETECT AND IDENTIFY ALL LOW SPOTS AND/OR BELLIES. FAILURE TO RUN WATER PRIOR TO VIDEOTAPING WILL RESULT IN REJECTION OF THE VIDEO TAPE TESTING.
- 5. MANHOLE INSPECTION – CONTOUR FLOWLINE, BEDDING, STEPS, LIDS MARKED AND VENTED.
- 6. TO RELEASE SUBDIVISION FROM WATRRANTY PHASE, STORM DRAIN MUST BE RE–VIDEOED, ALL STORM DRAIN BOXES NEED TO BE CLEANED OUT FROM SWPPP AND CONSTRUCTION DEBRIS.

ROADWAYS

- 1. ROADWAY INSPECTION, DEPTH & COMPACTION.
- 2. BEFORE ROAD BASE PLACEMENT ALL UTILITY LINES MUST BE INSTALLED TO CITY STANDARDS.
- 3. ROAD BASE DENSITY TEST – DEPTH=12” MINIMUM & COMPACTION 95% (CONTRACTOR RESPONSIBLE TO GIVE ROAD BASE DENSITY RESULTS TO CITY – PRIOR TO ASPHALT PLACEMENT), (ALL COMPACTION TEST MUST HAVE CURRENT PROCTOR ON SOILS).
- 4. ALL NEW ROADS REQUIRE MAT/FABRIC TO BE INSTALLED UNDER ROADWAYS (SEE DETAIL).
- 5. ASPHALT PLACEMENT – DEPTH=3” OR MATCH EXISTING, COMPACTION TEST – 95% & TEMPERATURE 50 AND RISING.

CURB & GUTTER AND SIDEWALK

- 1. CURB & GUTTER AND SIDEWALK INSPECTION – DEPTH & COMPACTION.
- 2. WATER FLOW CURB & GUTTERS TO VERIFY SURFACE FLOW.
- 3. CURB & GUTTER AND SIDEWALK INSPECTION – AESTHETICS, CHIPS, CRACKS AND WORKMANSHIP WALK THROUGH.

\*ALL UTILITY TRENCHES WITHIN THE ROADWAYS MUST HAVE IMPORTED MATERIAL WITH UPDATED PROCTOR, UNLESS SPECIFIED BY PLAIN CITY REPRESENTATIVE

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REVIEW

FILE : CITY STANDARDS

JUB PROJ. # :----

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DESIGN BY: PJT

CHECKED BY: PJT

ONE INCH

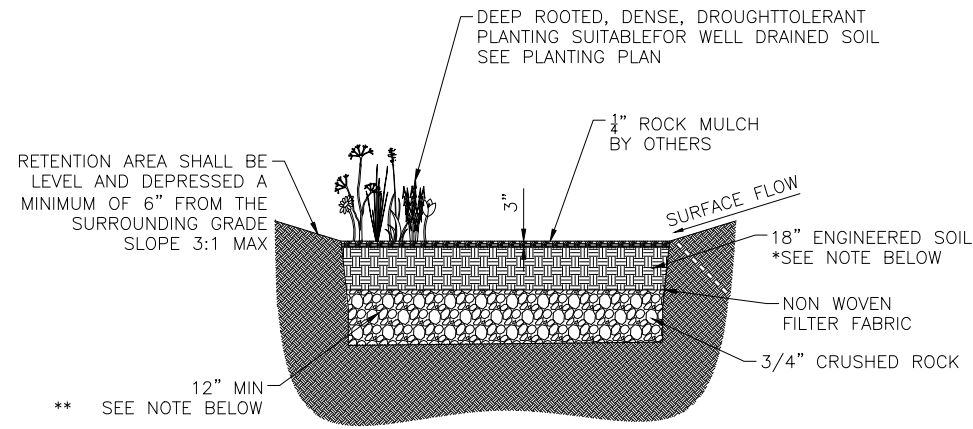
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TESTING AND INSPECTION	

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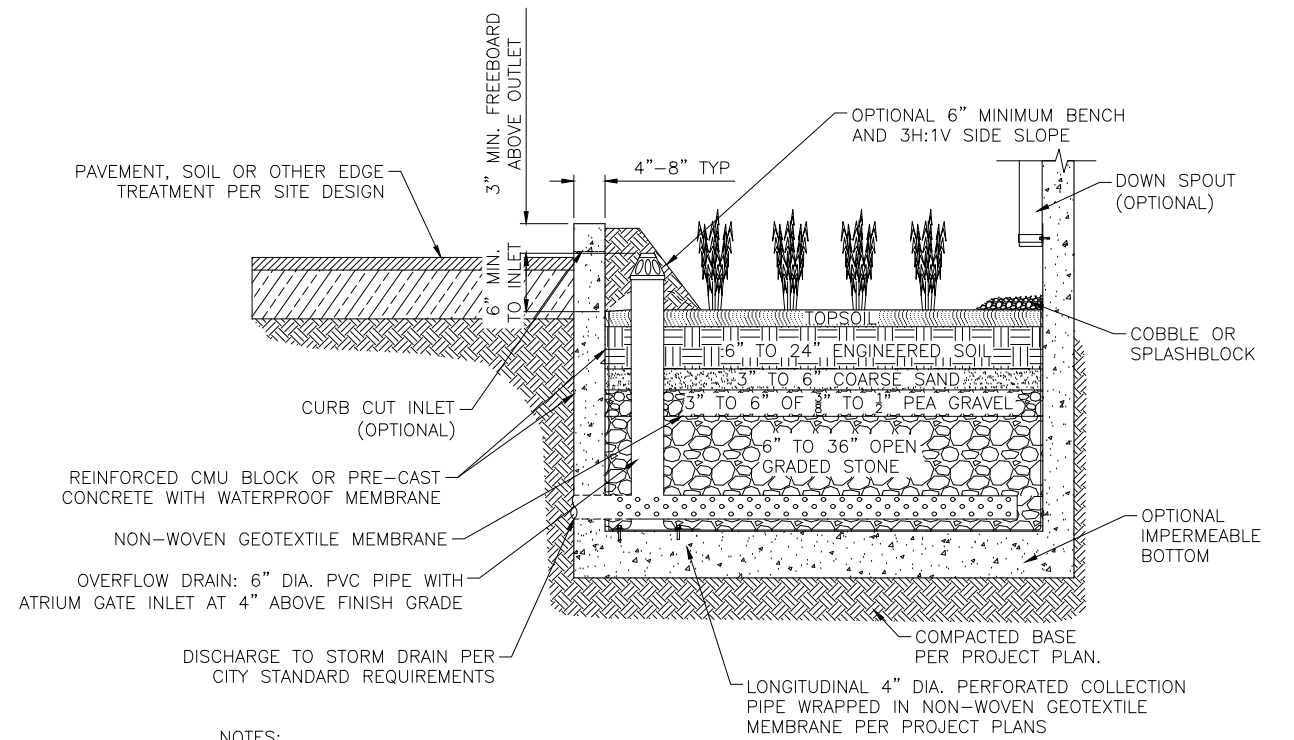
24



\*BIORETENTION "ENGINEERED SOIL" LAYER SHALL BE MINIMUM 18" DEEP "SANDY LOAM" SOIL MIX WITH NO MORE THAN 5% CLAY CONTENT. THE MIX SHALL CONTAIN 50-60% SAND, 20-30% COMPOST OR HARDWOOD MULCH, AND 20-30% TOPSOIL.

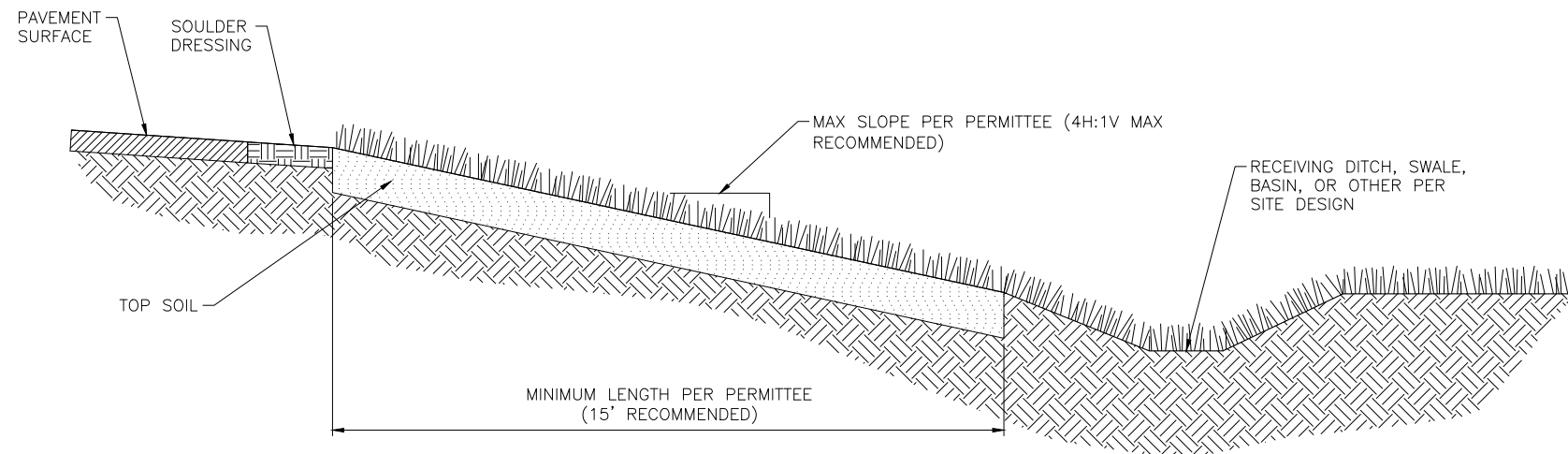
\*\*3/4" CRUSHED ROCK LAYER SHALL BE A MINIMUM 12" BUT MAY BE DEEPENED TO INCREASE THE INFILTRATION AND STORAGE ABILITY OF THE BASIN.

**1 RAIN GARDEN**  
SCALE: N.T.S.



- NOTES:
1. AT LEAST 9 INCHES SHALL BE PROVIDED BETWEEN THE PLANTING SURFACE AND THE CREST OF EACH PLANTER.
  2. PLANTERS SHALL NOT BE LOCATED ON UNEVEN OR SLOPED SURFACES.
  3. TOP SOIL/PLANTING MIX IS AT LEAST 18" DEEP.
  4. TOP SOIL CONTAINS NO MORE THAN 30% COMPOST.
  5. DIRECT OVERFLOW DISCHARGE CITY STANDARD REQUIREMENTS.
  6. SEE BIORETENTION CELL FACT SHEET FOR MORE INFORMATION.

**2 BIORETENTION CELL**  
SCALE: N.T.S.




- NOTES:
1. DIMENSIONS SHOWN MAY VARY BASED ON SITE CONDITIONS

**3 VEGETATED STRIP**  
SCALE: N.T.S.

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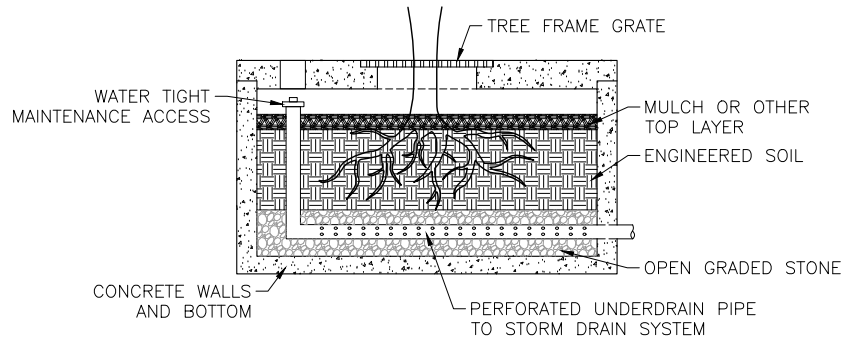
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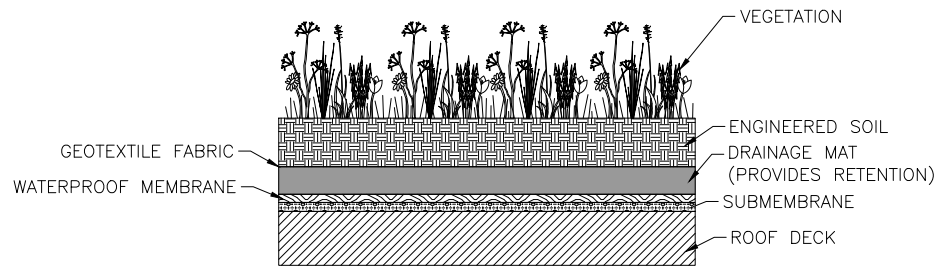
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25

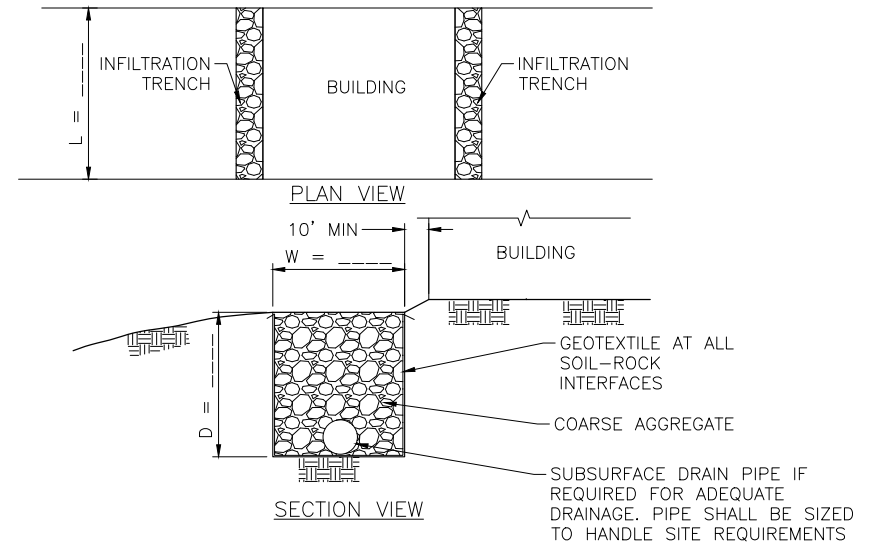


4 TREE BOX FILTER  
SCALE: N.T.S.



NOTES:  
1. DIMENSIONS SHOWN MAY VARY BASED ON SITE CONDITIONS

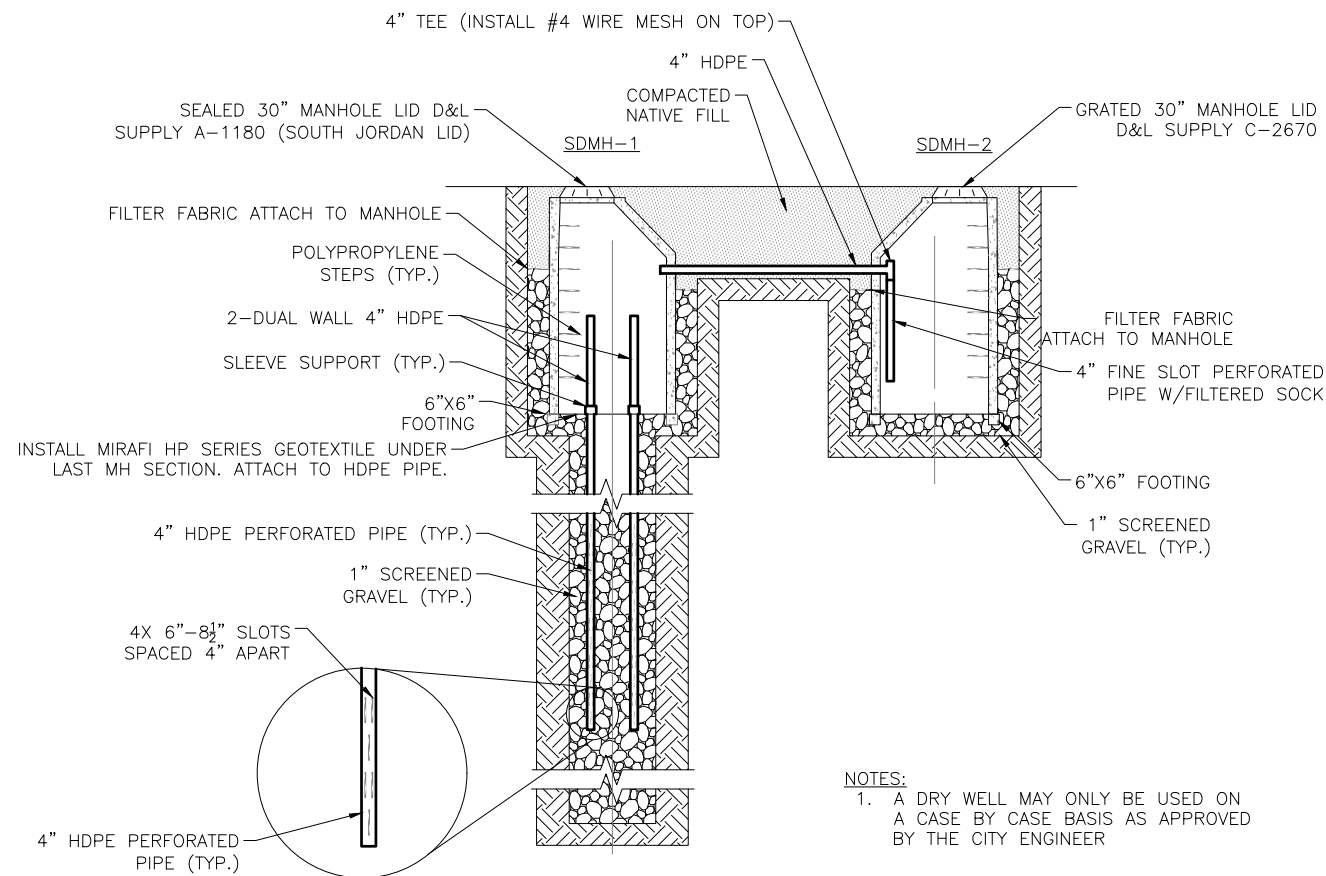
5 GREEN ROOF  
SCALE: N.T.S.



- NOTES:
1. COARSE AGGREGATES SHALL BE WELL GRADED 6" MINUS WITH NO MORE THAN 20% SMALLER THAN A #4 SIEVE.
  2. INFILTRATION TRENCH SHALL PROVIDE ADEQUATE STORAGE FOR THE DESIGN STORM.
  3. THE VOLUME OF RUNOFF WATER TEMPORARILY STORED IN THE TRENCH IS APPROXIMATELY 40% OF THE TOTAL VOLUME OF THE INFILTRATION TRENCH INCLUDING AGGREGATE.
  4. SUBSURFACE DRAIN PIPE REQUIRED UNLESS ADEQUATE SOIL PERMEABILITY CAN BE VERIFIED.
  5. GEOTEXTILE (NON-WOVEN, NEEDLE PUNCHED) MIN. CRITERIA:

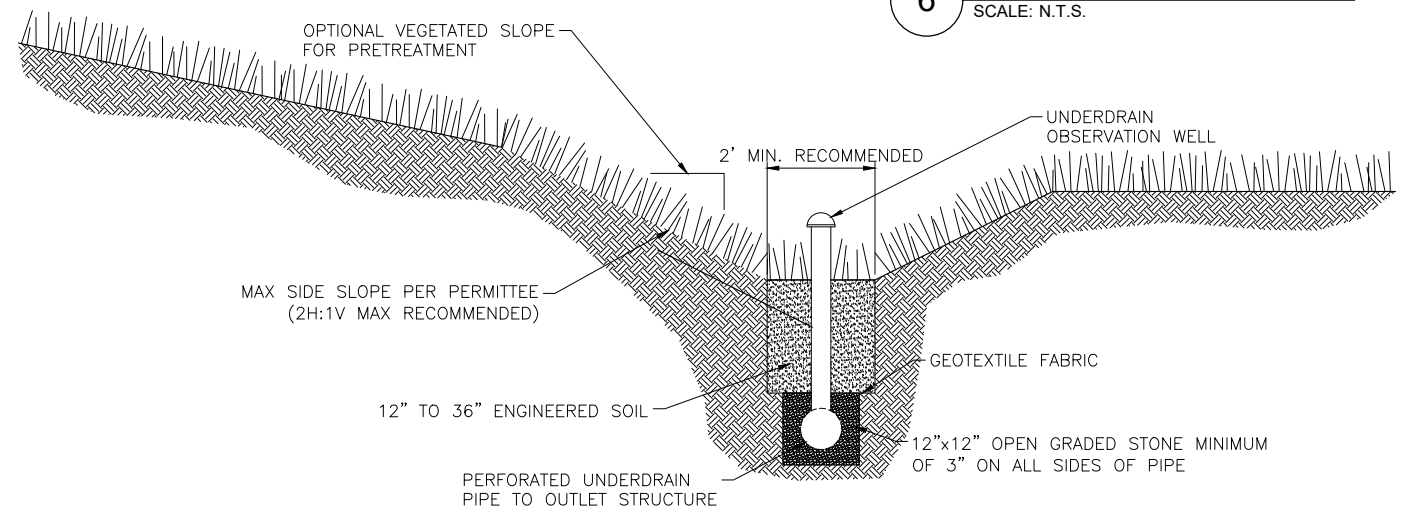
GRAB TEXTILE STRENGTH (LB) ASTM D 4632	202
ELONGATION AT FAILURE (%) ASTM D 4632	≥50
TRAPEZOIDAL TEAR STRENGTH (LB) ASTM D 6241	79
PUNCTURE STRENGTH (LB) ASTM D 6241	433
ULTRAVIOLET LIGHT (% RETAINED STRENGTH) ASTM 4355	MIN 50
APPARENT OPENING SIZE (AOS) ASTM D 4751	MAX 0.2 MM (US SIEVE SIZE 70)
PERMITTIVITY SEC <sup>-1</sup> ASTM D 4491	MIN 0.7
  6. GEOTEXTILE SPLICES SHALL OVERLAP 18 INCH MINIMUM.

6 INFILTRATION TRENCH  
SCALE: N.T.S.



- NOTES:
1. A DRY WELL MAY ONLY BE USED ON A CASE BY CASE BASIS AS APPROVED BY THE CITY ENGINEER

7 DRYWELL  
SCALE: N.T.S.




- NOTES:
1. ENGINEERED SOIL WILL IMPROVE FILTRATION
  2. UNDERDRAIN RECOMMENDED FOR LONGITUDINAL SLOPES <1%
  3. DIMENSIONS SHOWN MAY VARY BASED ON SITE CONDITIONS

8 BIOSWALE  
SCALE: N.T.S.

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SHEET NUMBER:  
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LAYER	MATERIAL TYPE*	MODERATE VEHICULAR		LIGHT VEHICULAR		PEDESTRIAN	
		GOOD SOILS**	POOR SOILS**	GOOD SOILS**	POOR SOILS**	GOOD SOILS**	POOR SOILS**
A	PERMEABLE UNIT PAVERS	3 1/8	3 1/8	3 1/8	3 1/8	3 1/8	3 1/8
B	GRAVEL LEVELING COURSE ASTM NO. 8	2	2	2	2	2	2
C	GRAVEL BASE COURSE ASTM NO. 57	6	6	6	4	4	4
D	GRAVEL RESERVOIR COURSE ASTM NO. 2, 3, OR 57	22	28	—	10	—	—

\* MATERIAL FINER THAN NO. 100 SIEVE SHALL NOT EXCEED 2 PERCENT FOR ANY GRAVEL LAYER.

\*\* "GOOD" AND "POOR" SOIL CLASSIFICATIONS BASED ON AASHTO GUIDE FOR DESIGN OF PAVEMENT STRUCTURES. SEE DESIGNER NOTES FOR SUBGRADE ASSUMPTIONS.

GAP WIDTH (IN)	JOINT FILLER AGGREGATE
3/8 OR 1/2	ASTM NO. 8 *
1/4	ASTM NO. 8/9 *
1/8	ASTM NO. 10 *

\* PROVIDED FOR REFERENCE ONLY, FOLLOW MANUFACTURER'S RECOMMENDATIONS

1. PAVEMENT SECTION MUST BE DESIGNED BY A LICENSED PROFESSIONAL CIVIL ENGINEER.
2. THICKER GRAVEL RESERVOIR COURSE (BEYOND STRUCTURAL DEPTH) AND SUBSURFACE CHECK DAMS MAY BE REQUIRED TO MEET PROJECT HYDROLOGIC PERFORMANCE GOALS.

1. UTILITY CONFLICTS SHALL BE MITIGATED PER CITY REQUIREMENTS. INCLUDE MEASURES TO PREVENT PREFERENTIAL FLOW INTO UTILITY TRENCHES (E.G., WATER STOP, TRENCH BLOCK, OR TRENCH COLLAR). PROPOSED UTILITY LINES TO BE LOCATED OUTSIDE OF FACILITY.
2. ABANDONED UTILITIES WITHIN FOOTPRINT OF FACILITY AND OBSERVED DURING CONSTRUCTION MUST BE REMOVED. COORDINATE WITH MUNICIPAL OR PRIVATE OWNER AND ENGINEER.

1. COMPACT SUBGRADE TO 90% OF MAXIMUM DENSITY PER STANDARD PROCTOR TEST (ASTM D698) FOR PEDESTRIAN APPLICATIONS, 95% OF MAXIMUM DENSITY PER MODIFIED PROCTOR TEST (ASTM D1557) FOR VEHICULAR APPLICATIONS, OR PER GEOTECHNICAL ENGINEER'S RECOMMENDATION.
2. PROOF-ROLL PREPARED SUBGRADE, REMOVE SOFT SPOTS, AND REPLACE WITH PERMEABLE STRUCTURAL FILL AS DIRECTED BY ENGINEER TO ACHIEVE UNIFORM SUBGRADE.
3. AFTER COMPACTION, SCARIFY SUBGRADE 1/4 TO 1/2 INCH DEEP BY HAND RAKE.
4. PLACE GEOTEXTILE, IF REQUIRED, ON SCARIFIED SUBGRADE.
5. PROVIDE FLOW DIVERSION AND EROSION CONTROL MEASURES TO PROTECT THE PERMEABLE PAVEMENT AREA FROM SEDIMENTATION UNTIL UPSTREAM CATCHMENT AREA IS THOROUGHLY STABILIZED.



EDGE TREATMENTS:	PC 1.1	PC 1.6
SUBSURFACE CHECK DAMS:	PC 2.1	PC 2.2
SUBSURFACE OUTLETS:	PC 3.1	PC 3.2

LAYER	MATERIAL TYPE*	MODERATE VEHICULAR		LIGHT VEHICULAR		PEDESTRIAN	
		GOOD SOILS**	POOR SOILS**	GOOD SOILS**	POOR SOILS**	GOOD SOILS**	POOR SOILS**
(A)	PERVIOUS CONCRETE	9	9.5	6.5	7	4.5	5
(B)	GRAVEL BASE COURSE ASTM NO. 3 OR 57	6	6	6	6	6	6
(C)	OPTIONAL GRAVEL RESERVOIR COURSE ASTM NO. 2, 3, OR 57	—	—	—	—	—	—

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3. SEE DESIGNER NOTES FOR SPECIFICATIONS, PP 1.1 THROUGH PP 1.2.

1. UTILITY CONFLICTS SHALL BE MITIGATED PER SAN FRANCISCO PUC SURFACE IMPROVEMENT STANDARDS [PENDING COMPLETION]. INCLUDE MEASURES TO PREVENT PREFERENTIAL FLOW INTO UTILITY TRENCHES (E.G., WATER STOP, TRENCH BLOCK, OR TRENCH COLLAR). PROPOSED UTILITY LINES TO BE LOCATED OUTSIDE OF FACILITY.
2. ABANDONED UTILITIES WITHIN FOOTPRINT OF FACILITY AND OBSERVED DURING CONSTRUCTION MUST BE REMOVED. COORDINATE WITH MUNICIPAL OR PRIVATE OWNER AND ENGINEER.

1. COMPACT SUBGRADE TO 90% OF MAXIMUM DENSITY PER STANDARD PROCTOR TEST (ASTM D698) FOR PEDESTRIAN APPLICATIONS, 95% OF MAXIMUM DENSITY PER MODIFIED PROCTOR TEST (ASTM D1557) FOR VEHICULAR APPLICATIONS, OR PER GEOTECHNICAL ENGINEER'S RECOMMENDATION.
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3. AFTER COMPACTION, SCARIFY SUBGRADE 1/4 TO 1/2 INCH DEEP BY HAND RAKE.
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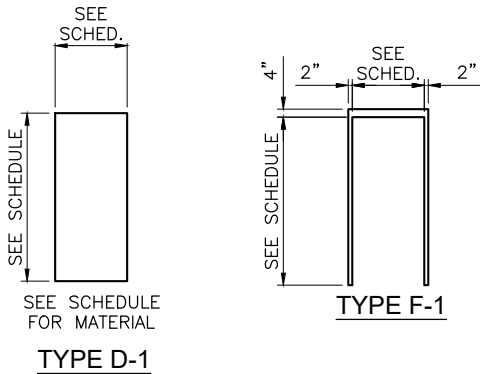




DOOR SCHEDULE

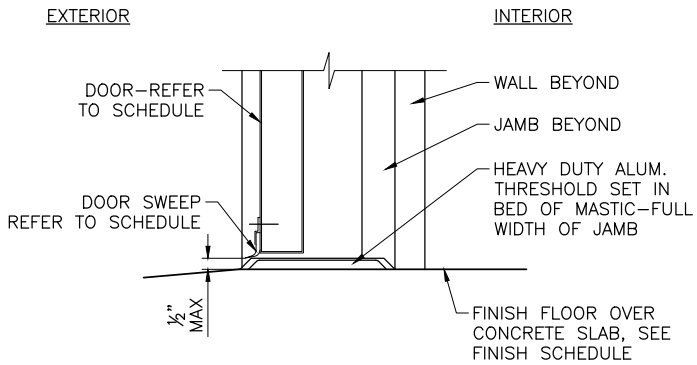
DOOR SCHEDULE																																										
TAG 000-X	DOOR SIZE			DOOR TYPE	DOOR MATERIAL	DOOR FINISH	OPENING DETAILS				FRAME DETAILS			FIRE RATING	COMMENTS	HINGES						LOCKSETS <sup>1</sup>						STOPS		DOOR SEAL			MISCELLANEOUS									
	WIDTH	HEIGHT	THICKNESS				HEAD	RIGHT JAMB	LEFT JAMB	SILL THRESHOLD	TYPE	MATERIAL	FINISH			NUMBER	PIVOTS	BALL BEARING	BRONZE / BRASS	STAINLESS STEEL	N.R. PINS	ENTRANCE	EXIT DEVICE	OFFICE	PASSAGE	PRIVACY	STORE ROOM	FLUSH BOLTS	DUST PROOF STRIKE	SURFACE BOLTS	WALL STOP	FLOOR STOP	SMOKE STRIP	WEATHER STRIP	ASTRAGAL	SWEEP	THRESHOLD	CLOSER	PUSH/PULL	COORDINATOR	KICK PLATE	ARMOR PLATE
101-A	2'-6"	7'-0"	1 3/4"	D-1	HM	P1	D2/A-002	D3/A-002	D3/A-002	D1/A-002	F-1	HM	P1	--	PAIR OF DOORS	6		X		X	X					X	X	X					X	X	X	X	X		X		X	X

- NOTES:
1. SECURE ALL DOOR HARDWARE SCREWS WITH "LOC-TIGHT" COMPOUND.
  2. KEYING INFORMATION: MATCH OWNERS MASTER KEYS AND CYLINDER CORE, COORDINATE WITH CONSTRUCTION MANAGER.



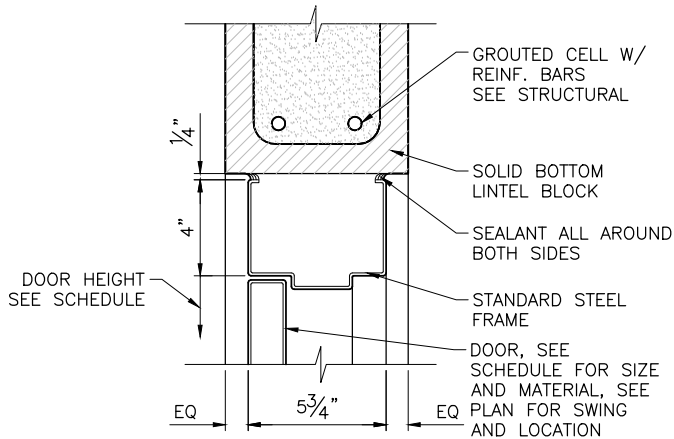
C4 DOOR & FRAME TYPES

SCALE: 1/4" = 1'-0"



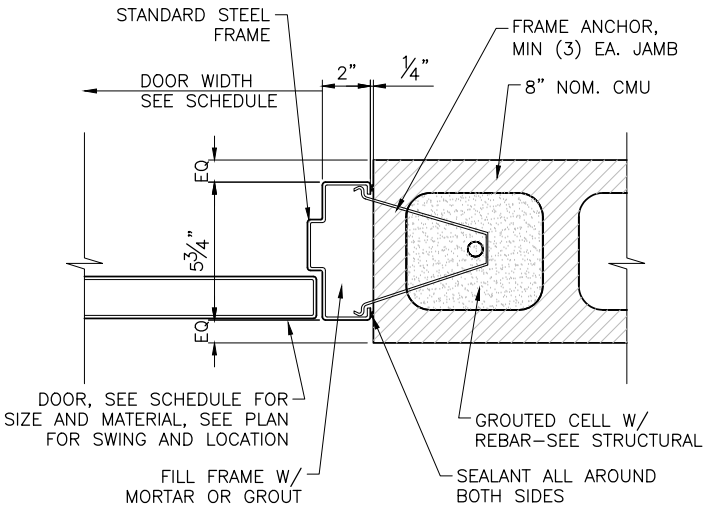
D1 DOOR THRESHOLD - CMU

SCALE: NOT TO SCALE



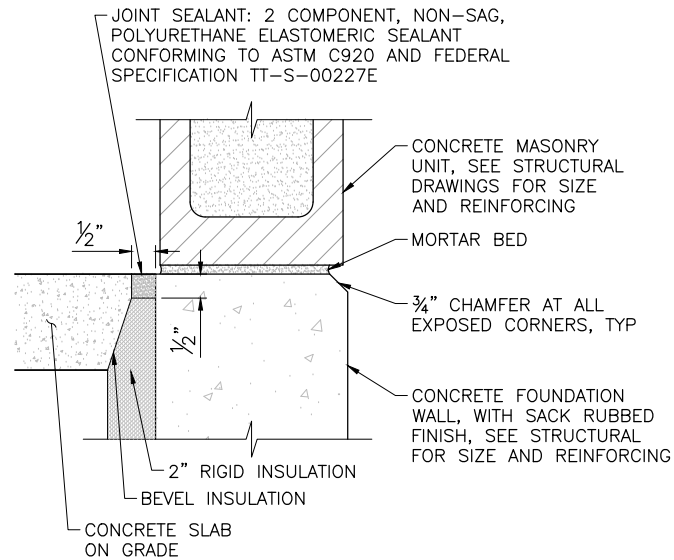
D2 DOOR HEAD - CMU

SCALE: NOT TO SCALE



D3 DOOR JAMB - CMU

SCALE: NOT TO SCALE



D4 CMU AT FOUNDATION WALL

SCALE: NOT TO SCALE

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File Name: 1/16/2020 - WAYSIDE PROJECT SUB PLAIN CITY STANDARDS 2020 CITY STANDARDS A-101X.DWG

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	DESIGN BY: PJT
	CHECKED BY: PJT
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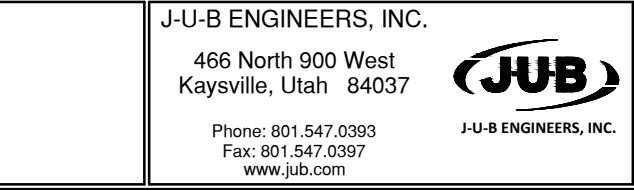
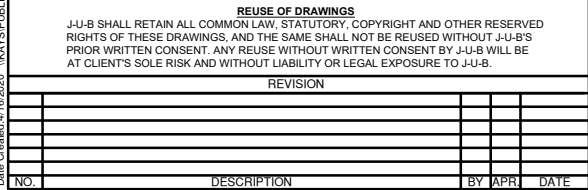
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TYPICAL LIFT STATION  
ARCHITECTURAL SCHEDULES


LAST UPDATED: 4/16/2020

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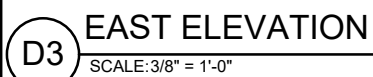
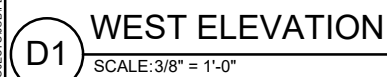
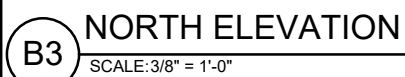
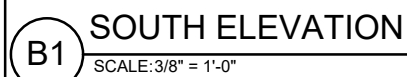
A-002



- ## KEY NOTES
- ① LINE OF ROOF ABOVE
  - ② SMITH POWER PRODUCTS 30kw 3 PHASE NATURAL GAS GENERATOR
  - ③ RADIATOR TRANSITION FLANGE
  - ④ FLEXIBLE DISCHARGE DUCT CONNECTION
  - ⑤ DUCTED EXHAUST
  - ⑥ AIR INLET VENTILATOR, 4'x4' "AMERICAN WARMING" EXTRUDED ALUMINUM LOUVER WITH BIRD SCREEN, AND 4'x4' "AMERICAN WARMING" AUTO DAMPER WITH EDGE AND END SEALS INTERLOCKED WITH GENERATOR TO OPEN AT START-UP
  - ⑦ RADIATOR DISCHARGE, 4'x4' "AMERICAN WARMING" EXTRUDED ALUMINUM LOUVER WITH BIRD SCREEN, AND 4'x4' "AMERICAN WARMING" AUTO DAMPER WITH EDGE AND END SEALS INTERLOCKED WITH GENERATOR TO OPEN AT START-UP
  - ⑧ DAMPER ACTUATOR
  - ⑨ GENERATOR STARTUP BATTERY
  - ⑩ NATURAL GAS METER
  - ⑪ STANDING SEAM METAL ROOF SYSTEM, OVER 30# FELT, MECHANICALLY FASTENED TO ROOF SHEATHING
  - ⑫ LINE OF EXTERIOR WALL BELOW
  - ⑬ METAL RIDGE CAP
  - ⑭ METAL VENTED SOFFIT & FASCIA MECHANICALLY FASTENED TO STRUCT
  - ⑮ SHADED AREA INDICATES: ICE & WATERSHIELD WATER PROOFING MEMBRANE, EXTEND 3'-0" MIN. FROM INTERIOR SIDE OF EXTERIOR WALL
  - ⑯ SLANT BACK ROOF VENT, SEE (D1/A-501)
  - ⑰ CONCRETE FOOTING, SEE STRUCTURAL DRAWINGS
  - ⑱ CONCRETE FOUNDATION WALL, SEE STRUCTURAL DRAWINGS
  - ⑲ CONCRETE MASONRY WALL, SEE STRUCTURAL DRAWINGS
  - ⑳ CONCRETE SLAB ON GRADE, SEE STRUCTURAL DRAWINGS
  - ㉑ 4" GRAVEL BASE
  - ㉒ PRE-ENGINEERED WOOD TRUSS, SEE STRUCTURAL DRAWINGS
  - ㉓ 6'-0" x 4'-0" CONCRETE LANDING PAD x 6" THICK, SEE CIVIL
  - ㉔ LOOSE FILL INSULATION IN ALL UNGROUTED CELLS, TYP
  - ㉕ DOOR AND FRAME, SEE SCHEDULE ON (A-002)

WALL TYPE	
	EXTERIOR CMU MASONRY WALL: REFER TO EXTERIOR ELEVATIONS FOR COLOR AND FINISH. RUNNING BOND, REFER TO STRUCTURAL FOR SIZE AND REINFORCING, WITH INTEGRAL WATER REPELLENT IN BLOCK AND MORTAR. EXTERIOR CLEAR SEALER AND INTERIOR PAINTED, SEE FINISH SCHEDULE.

	PUBLIC WORKS STANDARDS PLAIN CITY CORPORATION	LAST UPDATED: 4/16/2020  SHEET NUMBER:  <b>A-101</b>
	TYPICAL LIFT STATION ARCHITECTURAL FLOOR PLANS	



- ① LINE OF FOOTING AND FOUNDATION, SEE STRUCTURAL DRAWINGS
- ② ARCHITECTURAL CONCRETE FOUNDATION WALL
- ③ EXTERIOR CONCRETE FLATWORK, SEE CIVIL DRAWINGS
- ④ SPLIT-FACE CONCRETE MASONRY (FIELD). REFER TO WALL TYPES ON FLOOR PLAN.
- ⑤ ROOF WITH STANDING SEAM SYSTEM, SEE ROOF PLAN
- ⑥ SIDING WITH STANDING SEAM SYSTEM
- ⑦ SLANT BACK ROOF VENT
- ⑧ METAL VENTED SOFFIT AND FACIA (MBCI FLUSH SEAM PANEL SYSTEM OR APPROVED EQUAL) MECHANICALLY FASTEN TO STRUCTURE
- ⑨ DOOR AND FRAME, REFER TO DOOR SCHEDULE
- ⑩ APPROXIMATE FINISHED GRADE, SEE CIVIL DRAWINGS
- ⑪ LOUVER, SEE HVAC DRAWINGS
- ⑫ EXTERIOR LIGHT, SEE ELEC DRAWINGS

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AGENCY

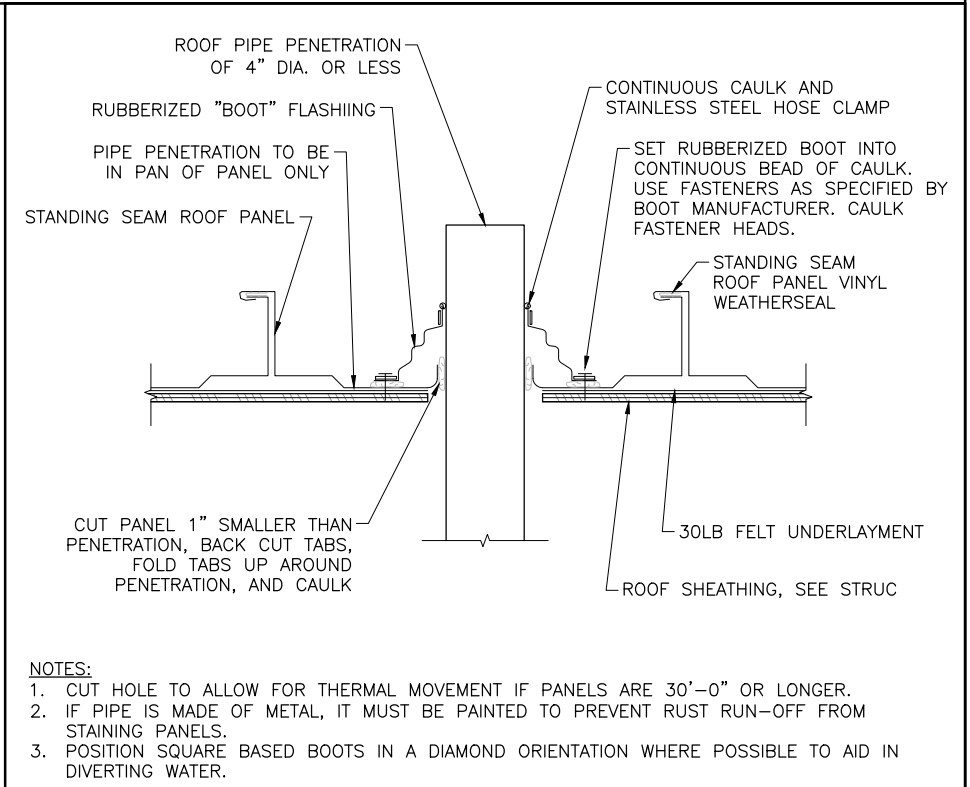
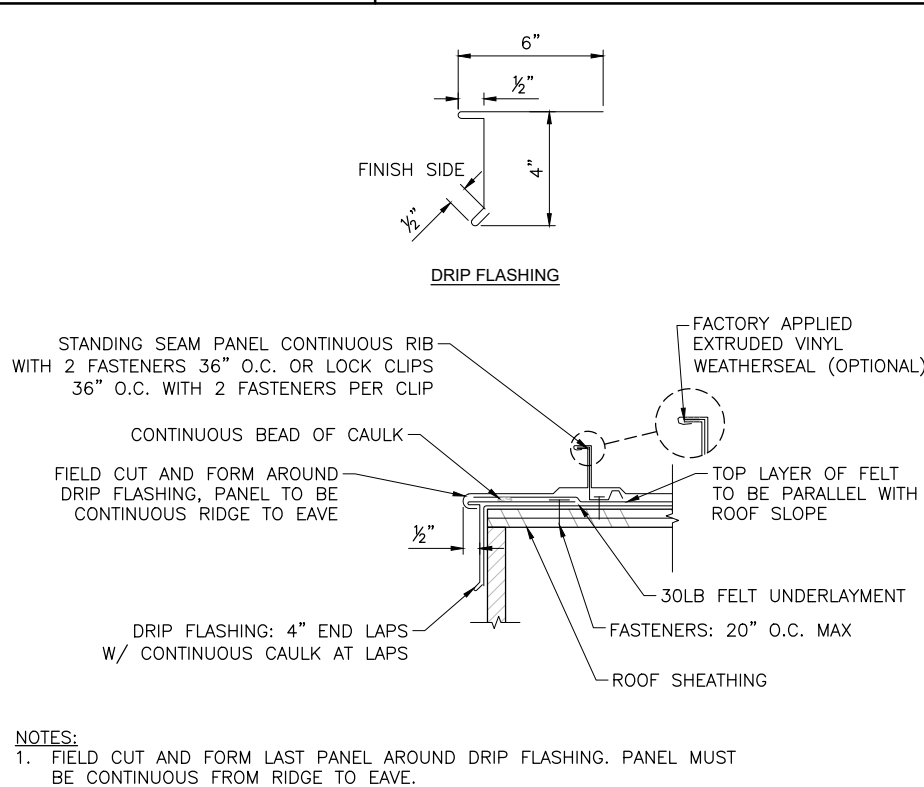
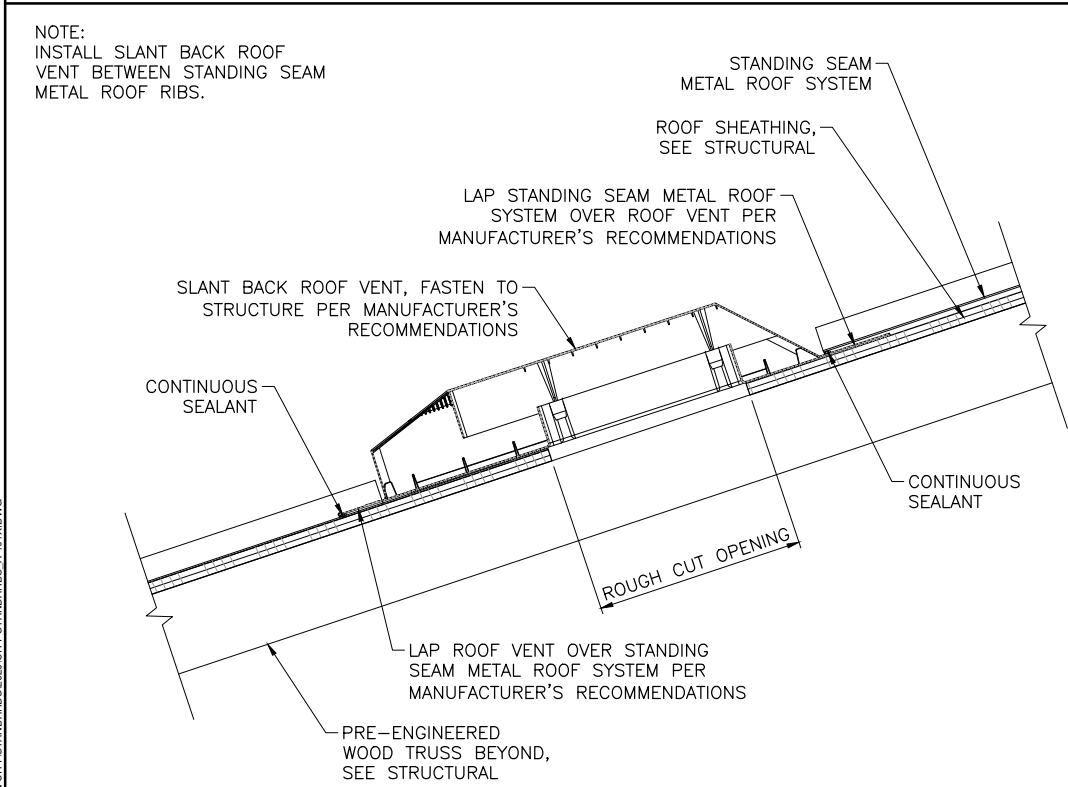
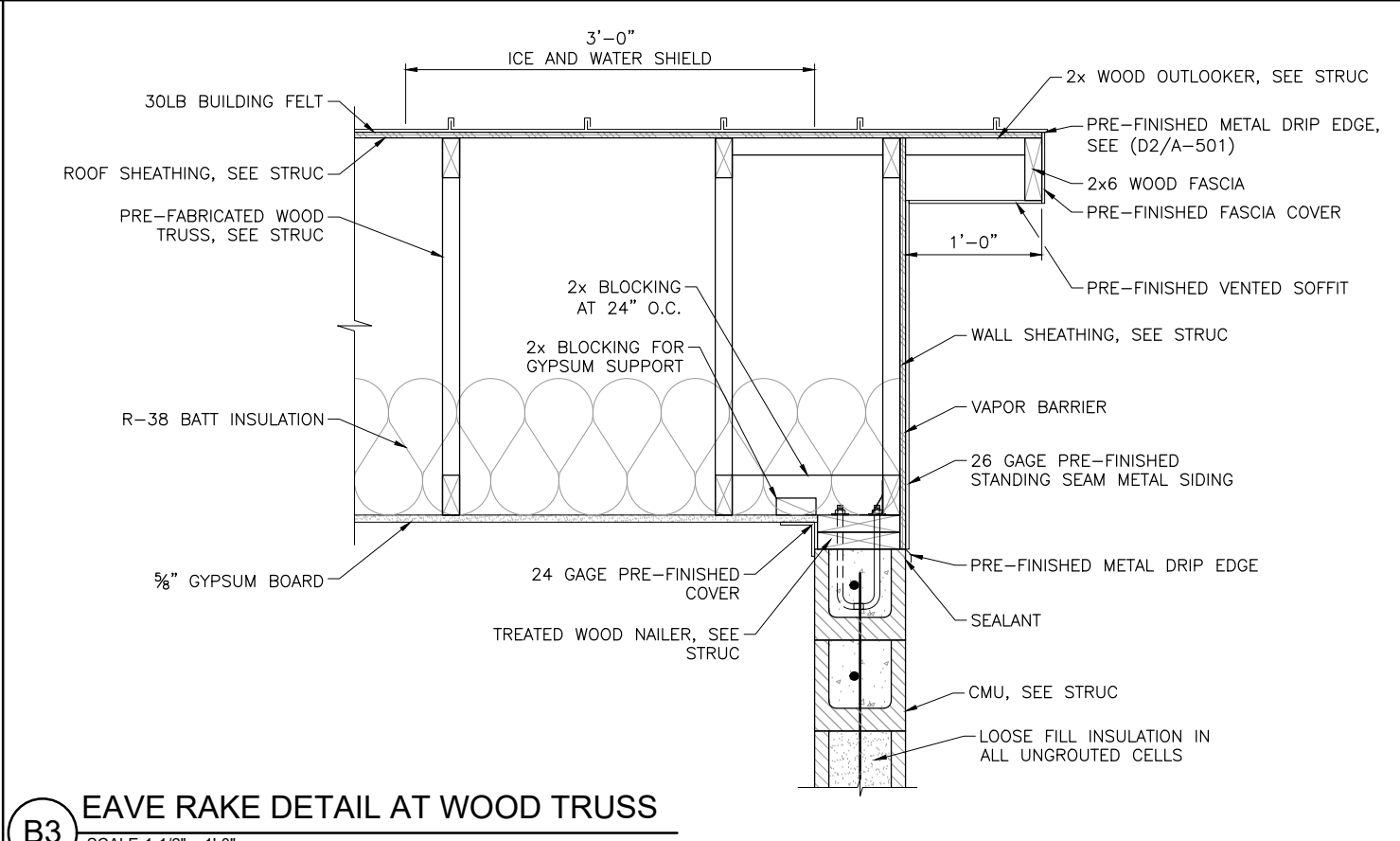
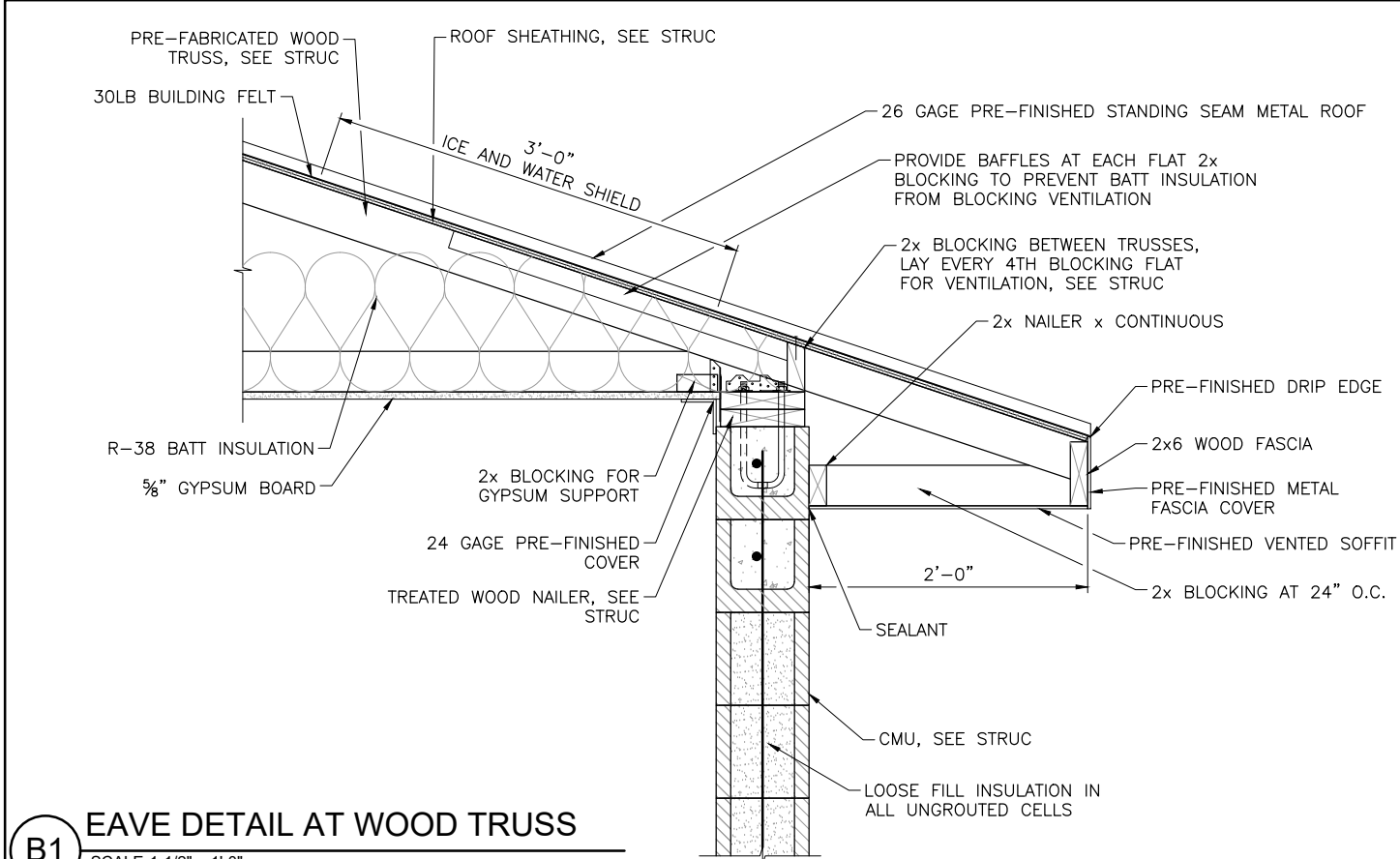
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TYPICAL LIFT STATION  
ARCHITECTURAL ELEVATIONS

SHEET NUMBER:

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 Date Created: 1/16/2020 10:45 AM Project: JUB PLAIN CITY STANDARDS A-501.DWG

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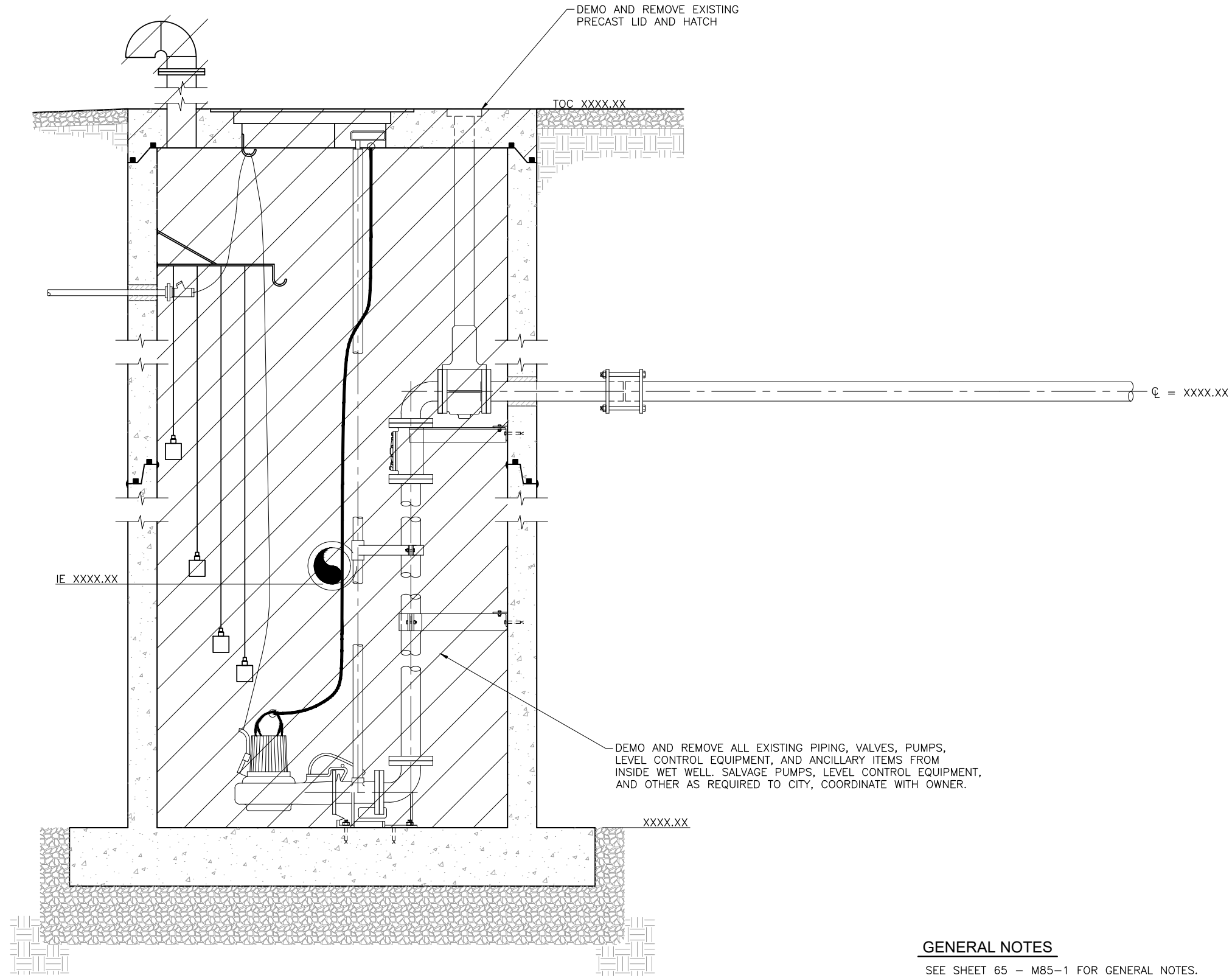
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PLAIN CITY CORPORATION**

TYPICAL LIFT STATION  
ARCHITECTURAL DETAILS

**A-501**

LAST UPDATED: 4/16/2020  
SHEET NUMBER:





**GENERAL NOTES**

SEE SHEET 65 – M85–1 FOR GENERAL NOTES.


**KEYED NOTES**

SEE SHEET 65 – M85–1 FOR KEY NOTES.

**D1 DEMOLITION SECTION VIEW**  
SCALE: 1"=1'-0"

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TYPICAL LIFT STATION  
DEMOLITION - SECTION VIEW

LAST UPDATED: 2/21/2020  
SHEET NUMBER:  
**M-102**



CONSTRUCTION NOTES:

1. PUMP STATION MECHANICAL EQUIPMENT SHOWN IS SCHEMATIC. CONTRACTOR IS RESPONSIBLE FOR CORRECT QUANTITIES AND PIPE SPOOL LENGTHS AS REQUIRED FOR A FULLY FUNCTIONAL INSTALLATION. VERIFY ALL DIMENSIONS (BOTH VERTICAL AND HORIZONTAL). VERIFY MANUFACTURERS CONNECTION DETAILS AND INSTALLATION REQUIREMENTS. PROVIDE A DIMENSIONED DRAWING SHOWING ALL VALVES, FITTINGS, PIPE SPOOLS, AND PUMP CONNECTIONS WITH SHOP DRAWING SUBMITTAL. COORDINATE ALL WORK WITH RELATED TRADES TO AVOID CONFLICTS. CONFIGURATION MAY VARY DEPENDING UPON SITE.
2. REUSE EXISTING FLOAT SWITCHES AND MATCH EXISTING LEVEL CONTROL SETTINGS (FIELD ADJUST TO OPTIMIZE PERFORMANCE AS REQUIRED):
3. INSTALL PUMP AND ALL RELATED PUMP EQUIPMENT IN STRICT ACCORDANCE WITH THE DRAWINGS, SPECIFICATIONS, AND MANUFACTURERS RECOMMENDATIONS.
4. PROTECT BUILDINGS, FENCES, CURBS, AND SIDEWALKS ADJACENT TO THE SITE, UNLESS NOTED OTHERWISE. DAMAGE BY CONTRACTORS OPERATIONS SHALL BE REPAIRED AT CONTRACTORS EXPENSE.
5. UNLESS NOTED OTHERWISE, ALL PIPING AND FITTINGS FROM THE PUMPS THROUGH THE VALVE VAULT SHALL BE DUCTILE IRON CLASS 53 AND HAVE A 2-PART HIGH BUILD COAL TAR EPOXY COATING (40 MIL THICKNESS) ON INTERIOR AND EXTERIOR SURFACES.
6. ALL BOLTS, NUTS, WASHERS, FASTENERS, ETC. SHALL BE STAINLESS STEEL, TYPE 304 OR 316.
7. INSTALL FLEXIBLE SLEEVE-TYPE COUPLINGS ADJACENT TO CONCRETE STRUCTURES.
8. NOT ALL FEATURES ARE SHOWN IN BOTH PLAN AND SECTION VIEWS FOR CLARITY.

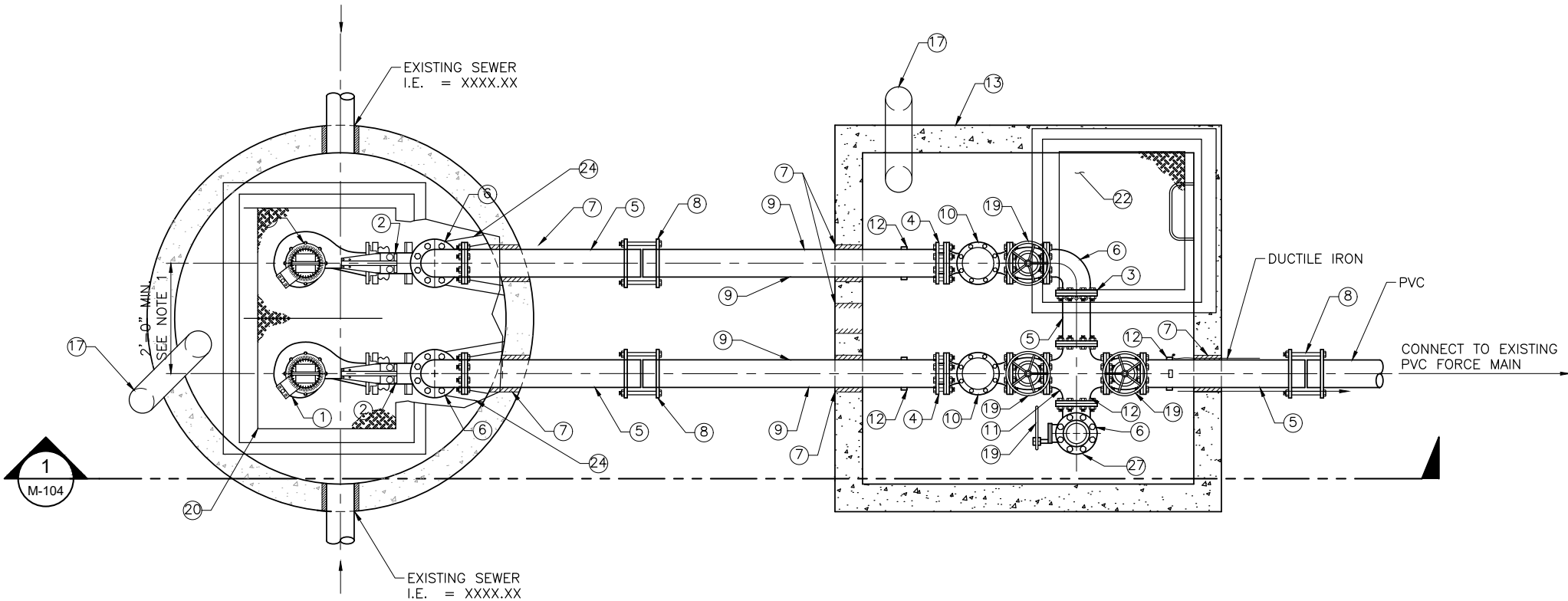
KEYED NOTES:

1. 5HP FLYGT SUBMERSIBLE PUMP MODEL 3102, PROVIDED BY OWNER AND INSTALLED BY CONTRACTOR.
2. 4" PUMP QUICK DISCONNECT DISCHARGE ELBOW AND MOUNTING BASE WITH EPOXY-SET ST.ST. ANCHOR BOLTS, VERIFY SIZE WITH PUMP MFTR. PROVIDED BY OWNER AND INSTALLED BY CONTRACTOR.
3. 4" UNI-FLANGE (MEGA-LUG)
4. 4" RESTRAINED FLANGE COUPLING ADAPTER
5. 4" DI PIPE SPOOL (FLxPE)
6. 4" DI 90° LONG RADIUS ELBOW (FLGxFLG)
7. CORE DRILL AND LINK SEAL, SEE DETAIL. FIELD VERIFY AND MATCH CONNECTION ELEVATION.
8. 4" FLEXIBLE SLEEVE-TYPE PIPE COUPLING
9. 4" DI PIPE SPOOL (PExPE)
10. 4" SWING CHECK VALVE (FLGxFLG)
11. 4"x4" DI CROSS (FLG)
12. VALVE/PIPE SUPPORT, SEE DETAIL
13. 6'-0"x6'-0"x6'-6" HIGH PRECAST CONCRETE VALVE VAULT WITH PLASTIC COATED STEPS. 6" MIN. WALL THICKNESS. HS-20 RATED. MODIFY OPENINGS AS REQ'D TO ACCOMMODATE PIPING AND ACCESS HATCH AS SHOWN. COAT EXTERIOR WITH WATERPROOFING TREATMENT.
14. 6FT DIAMETER HS-20 RATED PRECAST LID WITH SCREENED VENT AND ACCESS HATCH. FIELD VERIFY DIMENSIONS.
15. EXISTING 6FT DIAMETER PRECAST CONCRETE WETWELL.
16. STAINLESS STEEL PUMP REMOVAL SYSTEM, COMPLETE WITH MOUNTING BRACKETS AND INTERMEDIATE SUPPORT BRACES. PROVIDED BY OWNER AND INSTALLED BY CONTRACTOR.
17. SCREENED VENT, SEE DETAIL
18. GROUT PLUG (WATER TIGHT)
19. 4" PLUG VALVE WITH HAND WHEEL (FLGxFLG)
20. 30"x48" MIN. ALUMINUM DOUBLE LEAF ACCESS DOOR WITH STAINLESS STEEL HARDWARE AND ORANGE SAFETY GRATE - OPENING DIMENSIONS AND DOOR LOCATION SHALL BE IN ACCORDANCE WITH PUMP MANUFACTURERS REQUIREMENTS. HATCH SHALL BE HS-20 TRAFFIC RATED AND WATER-TIGHT. PROVIDE RECESSED, LOCKABLE HASP COVERED WITH HINGED LID FLUSH WITH SURFACE. INSTALL DOOR SUCH THAT ENTRY SYSTEM IS NOT IN CONFLICT WITH DOOR.
21. STAINLESS STEEL CABLE SUPPORT BRACKET FOR POWER CABLES AND FLOAT SWITCHES, THE BRACKET IS SHOWN SCHEMATICALLY IN THESE DRAWINGS. THE SUPPORT BRACKET NEEDS TO BE LOCATED SO THE CABLES AND FLOATS ARE EASILY ACCESSIBLE FROM THE ACCESS HATCH. FIELD ADJUST TO AVOID CONFLICTS WITH PUMP REMOVAL AND TO OPTIMIZE FLOAT SWITCH PERFORMANCE.
22. 30"x30" MIN. ALUMINUM SINGLE LEAF ACCESS DOOR WITH STAINLESS STEEL HARDWARE. HATCH SHALL BE HS-20 TRAFFIC RATED AND WATER-TIGHT. PROVIDE RECESSED, LOCKABLE HASP COVERED WITH HINGED LID FLUSH WITH SURFACE. INSTALL DOOR SUCH THAT ENTRY SYSTEM IS NOT IN CONFLICT WITH DOOR.
23. STAINLESS STEEL LIFTING CHAIN OR CABLE (MIN. STRENGTH 6,000 LBS.) WITH S.S. CLEVIS FITTING AT EACH END. PROVIDED BY OWNER AND INSTALLED BY CONTRACTOR.
24. THRUST RESTRAINT PIPE SUPPORT, SEE DETAIL.
25. MANHOLE JOINT WITH EXTRUDED BUTYL RUBBER SEAL OR EQUIVALENT. GROUT JOINT INSIDE AND OUT OR VULLCEM 16 JOINT SEALANT CAULKING, TYP.
26. CRUSHED AGGREGATE (¾" MINUS) COMPACTED TO 95% ASTM D-698 OR MODIFIED PROCTOR UNLESS INDICATED OTHERWISE IN GEOTECH REPORT.
27. 4" ALUM. LOCKING CAM-LOCK FITTING WITH PRESSURE CAP.

NOTE

THE LOCATION OF ALL EXISTING UNDERGROUND UTILITIES IS SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK. HE AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY HIS FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES

CALL 48 HOURS BEFORE YOU DIG  
811



1 MECHANICAL PLAN  
SCALE: N.T.S.

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TYPICAL LIFT STATION  
MECHANICAL PLAN

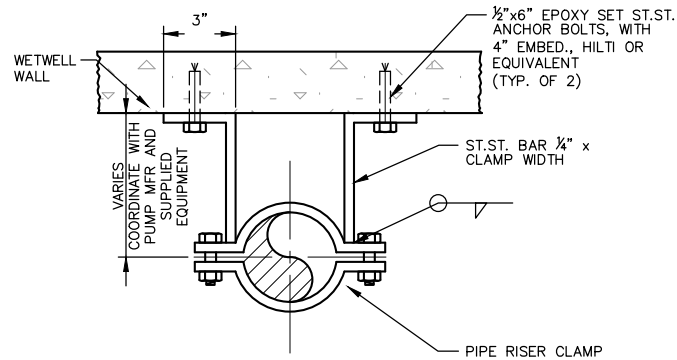
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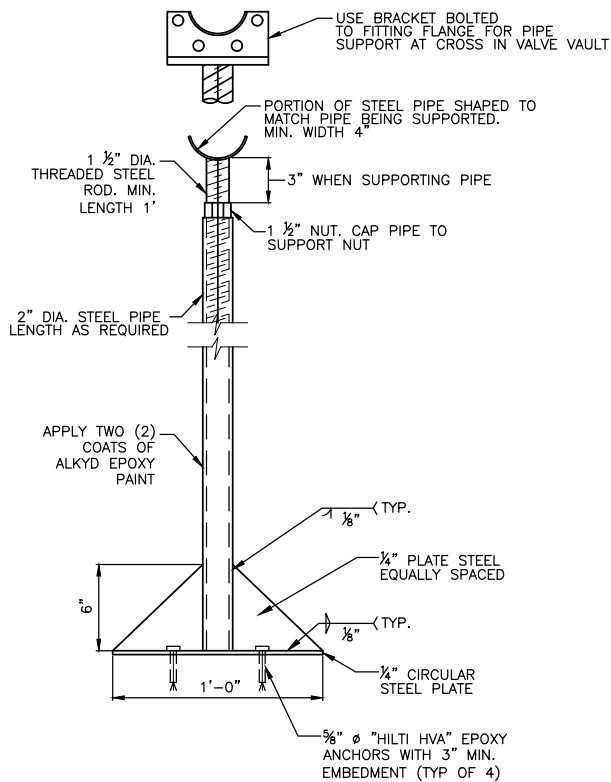
PLAN VIEW

**NOTES:**

1. MATERIALS TO BE STAINLESS STEEL
2. USE WHERE REQUIRED THRUST RESTRAINT IS MINIMAL.

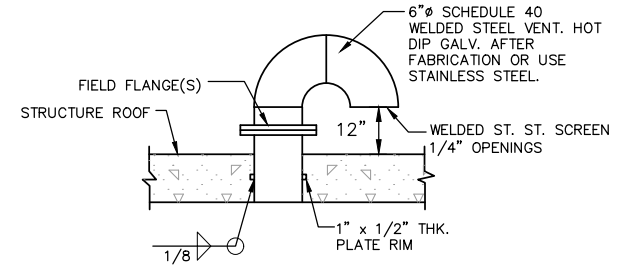
**1 PIPE BRACE DETAIL**

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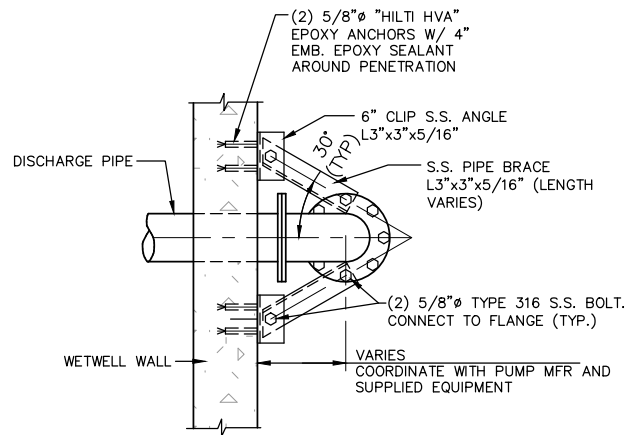
**2 TYP. STEEL PIPE SUPPORT DETAIL**

SCALE:N.T.S.



**3 SCREENED VENT DETAIL**

SCALE:N.T.S.



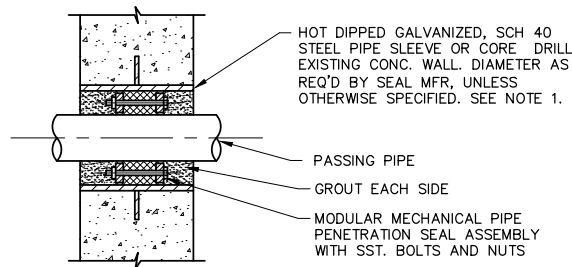
PLAN VIEW

**NOTES:**

1. MATERIALS TO BE STAINLESS STEEL
2. TOP SUPPORT MUST PROVIDE THRUST RESTRAINT.

**4 THRUST RESTRAINT PIPE SUPPORT DETAIL**

SCALE:N.T.S.



**NOTES:**

1. PVC SLEEVE WITHOUT SEEP COLLAR MAY BE ALLOWED IN SPECIFIC INSTANCES WITH ENGINEERS WRITTEN APPROVAL IN NON-WATER CONTAINING STRUCTURES, OR WHERE GROUNDWATER IS NOT OF CONCERN.

**5 MECHANICAL SEAL DETAIL**

SCALE:N.T.S.

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DESIGN BY: PJT

CHECKED BY: PJT

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PLAIN CITY CORPORATION

TYPICAL LIFT STATION  
DETAILS

LAST UPDATED: 2/21/2020

SHEET NUMBER:

M-105

Plot Date: 6/16/2020 1:45 PM Printed By: Daniel Johnson  
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GENERAL STRUCTURAL NOTES

1. GENERAL
- A. These general structural notes and specifications supplement the project written technical specifications and the project structural drawings.
- B. The contractor is responsible for all construction bracing, temporary shoring, and other site safety controls required during construction in accordance with all applicable local, state and federal regulations, to insure the stability and safety of all construction until it is completed and self-supporting.
- C. The contractor is responsible for all water, both above and below ground, runoff and other environmental controls required during construction to insure the site is maintained in compliance with all applicable local, state and federal regulations.
- D. Details on these plans are intended to depict the general construction details and methods for this structure. connection details and conditions not specifically shown that are similar in nature to those that are specified shall be assumed one and the same. if questions regarding the application of details are encountered, notify the engineer for clarification or instruction.
- E. Prior to implementing any changes to these plans, the engineer shall be notified in writing for their written approval. changes implemented without the engineers written approval shall relieve the engineer of any claim or liability resulting from that portion of the structure changed or affected by the change.
2. CONTRACTOR RESPONSIBILITY FOR COORDINATION
- A. It is the Contractors Prime responsibility to coordinate the work shown on all of the Project Drawings, general, special and technical specifications.
- B. The Contractor is responsible to verify all existing construction material types dimensions, elevations and conditions.
- C. The Contractor shall verify and coordinate the dimensions among all drawings and in the field prior to proceeding with any work or fabrication, any discrepancy shall be immediately reported to the Engineer.
- D. It is the Contractor's responsibility to carefully study and coordinate the construction requirements shown on the Architectural, Civil, Mechanical, Electrical, and the Structural Drawings. When conflicts or discrepancies are found between these plan sets and/or within these drawings, the Contractor shall report them immediately to the project Engineer for direction and/or clarification.
- E. Any construction work done by the Contractor before obtaining such clarification from the Project Engineer shall be at the Contractor's own risk and cost. Furthermore; any work required to correct, replace and/or restore the work as directed by the Engineer shall be at the Contractor's own risk and cost.
3. CODES
- A. International Building Code, IBC 2015 Edition.
- B. Minimum Design Loads for Buildings and Other Structures, ASCE 7; current edition.
- C. American Concrete Institute, ACI 318, Building Code Requirements for Structural Concrete; current edition.
- D. American Concrete Institute, ACI 530, Building Code Requirements and Specifications for Masonry Structures; current edition.
- E. American Concrete Institute, ACI 301, Specifications for Structural Concrete.
- F. National Design Specifications, NDS For Wood Construction; current edition.
4. SPECIAL INSPECTIONS. Special Inspections per IBC Chapter 17 are required for the following items: C indicates Continuous, P indicates Periodic.
- |                                     |  |           |
|-------------------------------------|--|-----------|
| A. Soils.                           | By Geotechnical Engineer.  | Frequency |
| A.1.                                | Site preparation:  | P         |
| A.2.                                | Fill material verification:  | C         |
| A.3.                                | Fill placement and compaction:   | C         |
| A.4.                                | Lift thickness:  | C         |
| B. Concrete.                        |  |           |
| B.1.                                | Reinforcement placement:   | P         |
| B.2.                                | Placement of cast-in-place anchors:  | P         |
| B.3.                                | Verification of use of required mix:   | C         |
| B.4.                                | Concrete placement:  | C         |
| B.5.                                | Verification of in-situ concrete prior to removal of forms and shores from elevated slabs:                       | P         |
| C. Post Installed Concrete Anchors. |  |           |
| C.1.                                | Installation:  | C         |
| D. Structural Masonry.              |  |           |
| D.1.                                | Verification of site proportioned mortar & grout:  | P         |
| D.2.                                | Observation of prism preparation:  | C         |
| D.3.                                | Placement of masonry units & mortar joints:  | P         |
| D.4.                                | Verification of size and location of structural elements:  | P         |
| D.5.                                | Anchorage of masonry to structural members and diaphragms including type, size and location of anchors           | P         |
| D.6.                                | Type, grade and size of reinforcing steel:   | P         |
| D.7.                                | Reinforcing steel and connector placement:   | P         |
| D.8.                                | Cold/Hot weather masonry protection:   | P         |
| D.9.                                | Verify use of grout mix design:  | P         |
| D.10.                               | Verify grout space is clean prior to grouting:   | C         |
| D.11.                               | Grout placement:   | C         |
| E. Wood.                            |  |           |
| E.1.                                | Fabrication of pre-fabricated structural elements:   | P         |
| E.2.                                | Material verification of structural panels and nails for diaphragms and shear walls with edge nailing:           | P         |
| E.3.                                | Verification of framing size at diaphragm and shear wall panel edges with edge nailing less than or equal to 4": | P         |
| F.                                  | All special inspection shall be performed by ICBO certified inspectors.  |           |
5. SUBMITTALS
- A. Submit required copies, four (4) minimum, of product or material design information to the Engineer for review for the following items:
- A.1. Concrete mix designs and admixtures.
- A.2. Non-shrink grout.
- A.3. Expansion bolts.

- A.4. Epoxy Anchors.
- A.5. Structural masonry grout and mortar mix designs.
- B. The following items to be designed by others are considered "Deferred Submittals". Deferred submittals shall be accompanied by design drawings, shop drawings and structural calculations, stamped and signed by a Professional Structural Engineer currently registered in the State of Utah.
- B.1. Pre-engineered and shop fabricated wood joists and trusses.
- C. Submit required copies of shop drawings, four (4) minimum, to the Engineer for review prior to fabrication of the following items:
- C.1. Reinforcing steel for all concrete.
- C.2. Reinforcing steel for masonry walls.
6. DESIGN CRITERIA
- |  |  |
|--|--|
| A. Floor Load                              |  |
| A.1. Floor Live Load                       | 150 psf                                |
| B. Roof Snow Load                          |  |
| B.1. Ground Snow Load                      | Pg = 43 psf                            |
| B.2. Flat Roof Snow Load                   | Pf = 33 psf                            |
| B.3. Importance Factor                     | Is = 1.10                              |
| B.4. Snow Exposure Factor                  | Ce = 0.9                               |
| B.5. Thermal Factor                        | Ct = 1.1                               |
| C. Wind Load                               |  |
| C.1. Basic Wind Speed                      | V = 120 mph                            |
| C.2. Wind Importance Factor                | Iw = 1.0                               |
| C.3. Wind Exposure                         | C                                      |
| D. Seismic Load                            |  |
| D.1. Occupancy Category                    | III                                    |
| D.2. Seismic Importance Factor             | Ieq = 1.25                             |
| D.3. Mapped Spectral Response Acceleration |  |
| D.3.1. Short Period Acceleration           | Ss = 1.45                              |
| D.3.2. 1-Second Acceleration               | S1 = 0.48                              |
| D.4. Site Class (Soil Profile)             | D                                      |
| D.5. Spectral Response Coefficients        |  |
| D.5.1. Short Period Acceleration           | Sds = 0.97                             |
| D.5.2. 1-Second Acceleration               | Sd1 = 0.49                             |
| D.6. Seismic Design Category               | D                                      |
| D.7. Basic Seismic Force Resisting System  | Special Reinforced Masonry Shear Walls |
| D.7.1. Response Modification Coef.         | R = 5.0                                |
| D.7.2. System Overstrength Factor          | Ωo = 2.5                               |
| D.7.3. Deflection Amplification Factor     | Cd = 3.5                               |
| D.8. Analysis Procedure                    | Equivalent Lateral Force               |
7. FOUNDATIONS
- A. All footings to be placed entirely on 24 inches of compacted structural fill. Proof roll sub-grade prior to placing structural fill where the material has been disturbed by the excavating equipment.
- B. All piers and footings outside or at the perimeter of the structure, or in other unheated areas shall be set to a depth of at least 30" below finish grade, unless other wise noted on the plans.
- C. Net allowable bearing pressure Qa = 1,500 psf.
- D. Local areas of soft and/or unacceptable material encountered at bottom of footing elevations indicated on the plans must be over-excavated and brought up to design grade with compacted "structural fill" or "lean concrete fill".
- E. See specifications for structural fill requirements.
- F. Design for the mitigation of subsurface water shall be the responsibility of the Contractor.
- G. The Engineer shall be notified in writing if any clay type soils, debris or unconsolidated materials are encountered during excavations for foundations.
8. CONCRETE
- A. GENERAL. Concrete shall be proportioned to provide an average compressive strength, fc, as prescribed in ACI 318 Section 26.4.3 and shall satisfy the durability criteria of ACI 318 Chapter 19.
- B. PROJECT CONCRETE MIX TYPES: Concrete shall be proportioned and furnished for the various project uses as indicated on the plans and as follows:
- B.1. M1: Footings: fc = 3,000 psi, Absolute water-cement ratio by weight = 0.45, Air Content = 4-6%.
- B.2. M2: Foundation walls, slab on grade, and all other miscellaneous concrete: fc = 4,000 psi, Absolute water-cement ratio by weight = 0.45, Air Content = 4-6%.
- C. CONCRETE MIX COMPONENTS.
- C.1. A water-reducing admixture conforming to ASTM C494, used in strict conformance with the manufacturer's instructions, shall be incorporated in all concrete mix designs. At Contractor's option, a high-range water-reducing (HRWR) admixture conforming to ASTM C494, Type F or G, may be used provided the total slump is less than 10".
- C.2. Fly-ash conforming to ASTM C618 Type F or C, may replace up to 20% of the cement content, provided that the mix strength is substantiated by test data.
- C.3. Cement: ASTM C150 Type II.
- C.4. Water: Clean & Potable.
- C.5. Air entraining agent: ASTM C260. Except where noted non-air entrained.
- C.6. Aggregate: 0.75-inch Maximum aggregate per ASTM C33. Unless noted otherwise.
- C.7. Mix Proportioning: ACI 211.1.
- D. CONCRETE ACCESSORIES:
- D.1. REINFORCING STEEL: Reinforcing steel shall conform to ASTM A615 Grade 60; #3 bars may be Grade 40.
- D.2. EXPANSION BOLTS: Bolts noted on the plans as Expansion Bolts shall be HILTI Kwik Bolt-II, stud anchors; size and embedment as noted on the drawings, installed per the manufacturers recommendations; or an approved equal.
- D.3. EPOXY SET BOLTS & REBAR: Bolts and reinforcing steel bars noted on the plans as Epoxy or Construction Adhesive Set Bolts or Rebar shall be set in place utilizing

- the SIMPSON SET High Strength Epoxy system; size and embedment as noted on the drawings, installed per the manufacturers recommendations; or an approved equal.
- E. CONCRETE PROPORTIONS. Concrete mix proportioning shall be in accordance with ACI 211.1; Standard Practice for Selecting Proportions for Normal, Heavyweight, and Mass Concrete.
- F. CONCRETE MIX VERIFICATION: Concrete mix designs shall be verified by standard 28-day cylinder tests per ASTM C39.
- G. EVALUATION AND ACCEPTANCE OF CONCRETE. Concrete shall be tested in accordance with the requirements of ACI 318 Section 26.12.
- H. MIXING & PLACING CONCRETE. Concrete shall be prepared, mixed, placed and consolidated in accordance with ACI 318 Section 26.5 and as follows:
- H.1. ACI 304; Guide for Measuring, Mixing, Transporting, and Placing Concrete.
- H.2. ACI 309; Guide for Consolidation of Concrete.
- I. CONCRETE CURING. Concrete shall be maintained above 50-degrees F and in a moist condition for at least 7 days after placement, except when cured in accordance with ACI 318 Section 26.5.3.
- I.1. Curing of concrete shall be per the recommendations given in ACI 308; Guide to Curing Concrete.
- J. COLD WEATHER REQUIREMENTS. Adequate equipment shall be provided for heating concrete materials and protecting concrete during freezing or near-freezing weather. The recommended procedures listed in ACI 306; Cold Weather Concreting shall be followed.
- J.1. Cold weather is defined as a period when, for more than 3 consecutive days, the following conditions exist:
- J.1.1. The average daily air temperature is less than 40-degrees F and
- J.1.2. The air temperature is not greater than 50-degrees F for more than one-half of any 24-hour period.
- K. HOT WEATHER REQUIREMENTS. During hot weather, proper attention shall be given to ingredients, production methods, handling, placing, protection, and curing to prevent excessive concrete temperatures or water evaporation that could impair required strength or serviceability of the member or structure. The recommended procedures listed in ACI 305; Hot Weather Concreting shall be followed.
- K.1. Hot weather is any combination of the following conditions that tends to impair the quality of freshly mixed or hardened concrete by accelerating the rate of moisture loss and rate of cement hydration, or otherwise causing detrimental results:
- K.1.1. High ambient temperature.
- K.1.2. High concrete temperature.
- K.1.3. Low relative humidity.
- K.1.4. Wind speed.
- K.1.5. Solar radiation.

9. FORMWORK AND FINISHING

- A. Forms shall result in a final structure that conforms to shapes, lines, and dimensions of the members as required by the design drawings and specifications.
- A.1. Design of formwork shall be in accordance with ACI 318 Section 26.11.1.
- A.2. Formwork shall be in accordance with ACI 347; Guide to Formwork for Concrete.
- B. Tolerances for finished concrete surfaces shall meet the following requirements from ACI 347, class of surface is per Table 3.1:
- B.1. Footings: Class C
- B.2. Foundation walls: Class B
- B.3. Above grade concrete not visible to sight: Class B
- B.4. Above-grade concrete visible to sight: Class A
- C. REMOVAL OF FORMS.
- C.1. Concrete forms shall not be removed until the retained concrete has reached the following minimum percentage of the required 28 day compressive strength:
- C.1.1. Footings and base slabs on grade: 50% of f'c.
- C.1.2. Foundation walls and columns: 67% of f'c.
- C.2. Where concrete cylinder tests are not available for strength verification the following guide may be used when permitted by the Project Engineer:
- C.2.1. Footings and base slabs on grade: 12 hours.
- C.2.2. Foundation walls and columns: 24 hours.
- D. EMBEDMENTS IN CONCRETE.
- D.1. Conduits, pipes, and sleeves of any material not harmful to concrete and within limitations of ACI 318 Sections 20.7 and 26.8 shall be permitted to be embedded in concrete with approval of the Project Engineer, provided they are not considered to replace structurally the displaced concrete.
- D.2. Conduits and pipes of aluminum shall not be embedded in structural concrete unless effectively coated or covered to prevent aluminum-concrete reaction or electrolytic action between aluminum and steel.
- E. CONSTRUCTION JOINTS.
- E.1. Construction joints shall only be placed where indicated on the project drawings or as approved by the Project Engineer.
- E.2. Construction joints shall be constructed in accordance with ACI 318 Section 26.5.6.
- E.3. Sawn contraction joints. Conform to ACI 301 Section 5.3.5.
- F. CONCRETE FINISHING. All concrete surfaces shall be finished in accordance with ACI 301.
- G. Formed Concrete Surfaces. After removal of forms, give each formed surface one or more of the following finishes:
- G.1. Non-liquid Retaining Concrete Structures:
- G.1.1. Concrete footings and foundations not exposed to view. Provide a surface finish SF-1.0 per Section 5.3.3.3a.
- G.1.2. Foundation wall and other surfaces below grade and not exposed to view. Provide a surface finish SF-2.0 per Section 5.3.3.3.b.
- G.1.3. Interior, exterior and top surfaces exposed to view to 6-inches below grade. Provide a surface finish per Section 5.3.3.3.c.
- G.2. Unformed Concrete Surfaces. Unformed concrete surfaces including the top surface of all concrete floor slabs shall be finished in accordance with ACI 301 Section 5.3.4 and ACI 302 Chapter 8.
- G.2.1. For the top surfaces of walls, provide a "Scratched finish" per Section 5.3.4.2.a.
- G.2.2. Interior floor surfaces shall receive a Troweled finish per Section 5.3.4.2c.
- G.3. Provide a Nonslip finish for exterior surfaces and where indicated on the plans.

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J-U-B ENGINEERS, INC.

AGENCY

FILE : CITY STANDARDS, S-001X
JUB PROJ. # :----
DRAWN BY: DTJ
DESIGN BY: PJT
CHECKED BY: PJT

ONE INCH

AT FULL SIZE, IF NOT ONE INCH, SCALE ACCORDINGLY

PUBLIC WORKS STANDARDS  
PLAIN CITY CORPORATION

TYPICAL LIFT STATION  
GENERAL STRUCTURAL NOTES

LAST UPDATED: 2/21/2020  
SHEET NUMBER:

S-001

## 10. DETAILS OF REINFORCEMENT

- ## 11. STRUCTURAL MASONRY REQUIREMENTS

- REUSE OF DRAWINGS**
- J-U-B SHALL RETAIN ALL COMMON LAW, STATUTORY, COPYRIGHT AND OTHER RESERVED RIGHTS OF THESE DRAWINGS, AND THE SAME SHALL NOT BE REUSED WITHOUT J-U-B'S PRIOR WRITTEN CONSENT. ANY REUSE WITHOUT WRITTEN CONSENT BY J-U-B WILL BE AT CLIENT'S SOLE RISK AND WITHOUT LIABILITY OR LEGAL EXPOSURE TO J-U-B.
- | REVISION |             |    |     |      |  |
|----------|-------------|----|-----|------|--|
|          |             |    |     |      |  |
| O.       | DESCRIPTION | BY | APR | DATE |  |

- J. ANCHOR BOLTS: Anchor bolts shall be accurately set with templates or by approved equivalent means and held in place to prevent movement. Conform to ACI 530.1, Section 3.4 D.
- K. WALL TIES: Install wall ties in accordance with ACI 530.1, Section 3.4 C.
- L. FOUNDATION DOWELS: It is the Contractor's responsibility to coordinate placement of dowels projecting from concrete foundations into reinforced masonry or brick walls.
- M. Bond beams with one (1) #5 bars horizontally shall be provided at all floor and roof lines and at the top of walls. Provide a bond beam with one (1) #5 bars horizontally above and below all openings, and extend these bars 2'-0" past the opening edge. Provide full height vertical reinforcement, matching typical vertical reinforcing, each side of openings, at wall ends and intersections.
- N. COLD-WEATHER CONSTRUCTION. When ambient air temperature is below 40-degrees F, implement Cold Weather procedures in accordance with ACI 530.1, Section 1.8 C.
- O. FIELD QUALITY CONTROL: Provide special inspection and verification in accordance with ACI 530.1, Section 3.7.
- P. CLEANING: Clean all exposed masonry surfaces in accordance with ACI 530.1, Section 3.8.

- A. LUMBER: Grading shall be according to the Standard Grading Rules of the WWPA. Typical structural lumber shall be Number 2 Douglas-Fir/Larch or better. Members noted as wood beams, posts or columns shall be Number 1 Douglas-Fir/Larch or better. Studs for interior non-bearing walls may be stud grade lumber. Lumber to be left exposed, without other finish and lumber in contact with concrete shall be pressure treated.
- B. TREATED LUMBER: Lumber, including wood sheathing, to be left exposed without other finish, located within 8" of finish grade, or in contact with concrete shall be pressure treated material. Contractor shall coordinate and verify that all steel items in contact with the treated material, including steel hangers, connectors and fasteners have a galvanized finish of sufficient thickness, or other type of protection, that is compatible with the specific treatment type selected.
- C. BOLTS & LAG SCREWS FOR WOOD CONSTRUCTION: Conform to ANSI/ASME Standards B18.2.1-1981 and the National Design Specification for Wood Construction (NDS) 1991 Edition Part VIII for Bolts and Part IX for Screws.
- D. WOOD SCREWS: Conform to ANSI/ASME Standards B18.6.1-1981 and the National Design Specification for Wood Construction (NDS) 1991 Edition Part XI.
- E. NAILS & SPIKES: Conform to Federal Specification FF-N-105B and the National Design Specification (NDS) 1991 Edition Part XII.
- F. NAILING: Where not otherwise specified on the plans, nailing shall conform to IBC Table 2304.10.1, Fastening Schedule. All nails shall be common wire nails or pneumatically driven nails with an equivalent cross-section and penetration, unless noted otherwise.
- G. LUMBER HARDWARE: Wood construction connectors shall be as manufactured by Simpson Strong-Tie Company; current catalog, or an approved equal. Hardware exposed to weather or view, in unheated portions of the structure, or as indicated on the drawings or in the specifications shall be hot-dipped galvanized with galvanized fasteners.
- H. ROOF SHEATHING: All roof sheathing shall be 7/16" nominal, Exterior APA rated Sheathing {24/16} installed with ply-clips.
- I. EXTERIOR WALL SHEATHING: All exterior wall sheathing shall be 1/2" nominal APA rated Exterior sheathing.
- J. All wood framing, blocking and nailing shall conform to the current local building code.
- K. All rafters, trusses and joists shall have full depth blocking, unless noted otherwise on the plans and details, at bearing supports, shear transfer supports, intermediate and cantilever supports and at mid-span, and as required by the building code or product supplier.
- L. All framing hardware including column caps and bases, joist hangers, truss anchors, straps, etc. shall be approved (i.e. Simpson Co. or equivalent) or custom fabricated specifically as detailed on the plans. They shall be installed with nails, screws or bolts exactly as called for by the manufacturer or as noted on the plans.
- M. WOOD SHEATHED ROOF DIAPHRAGMS:
  - M.1. Unless otherwise noted on the drawings, orient roof sheathing with face-grain perpendicular to supporting members, with joints in adjacent rows staggered 1/2 panel length.
  - M.2. Provide 2x4 flat blocking at unsupported panel edges in areas noted as "Blocked Roof Diaphragm".
  - M.3. Nail sheathing per roof sheathing schedule shown on drawings.
- N. WOOD SHEATHED SHEAR WALLS:
  - N.1. Shear wall sheathing to be oriented vertically.
  - N.2. All unsupported edges to be backed with 2x solid blocking.
  - N.3. Nail sheathing as shown on drawings.
  - N.4. Minimum nailing where not noted otherwise shall be 10d nails @ 6" o.c. to all panel edges and 12" o.c. at intermediate supporting members.

A. All pre-engineered/fabricated wood (PFT) trusses indicated on the drawings shall be metal press-plate connected wood trusses designed by a Professional Engineer registered in the State of Utah per these notes and the specifications.

B. Design PFT trusses to the following deflection limits:

B.1. Roof Dead + Live Load: Span/240 or 1 in maximum

B.2. Roof Live Load only: Span/360 or 1/2-in maximum


C. Design PFT trusses to support the concentrated and other distributed loads as shown on the plans in addition to the following loads:

C.1.	Dead Load (Top Chord) =	10 psf
C.2.	Dead Load (Bottom Chord) =	10 psf
C.3.	Snow Load* (Top Chord) =	33 psf
C.4.	Live Load** (Bottom Chord) =	10 psf
		53 psf Max Total Load

\* Does not include loads caused by drifting, unblanced or sliding snow

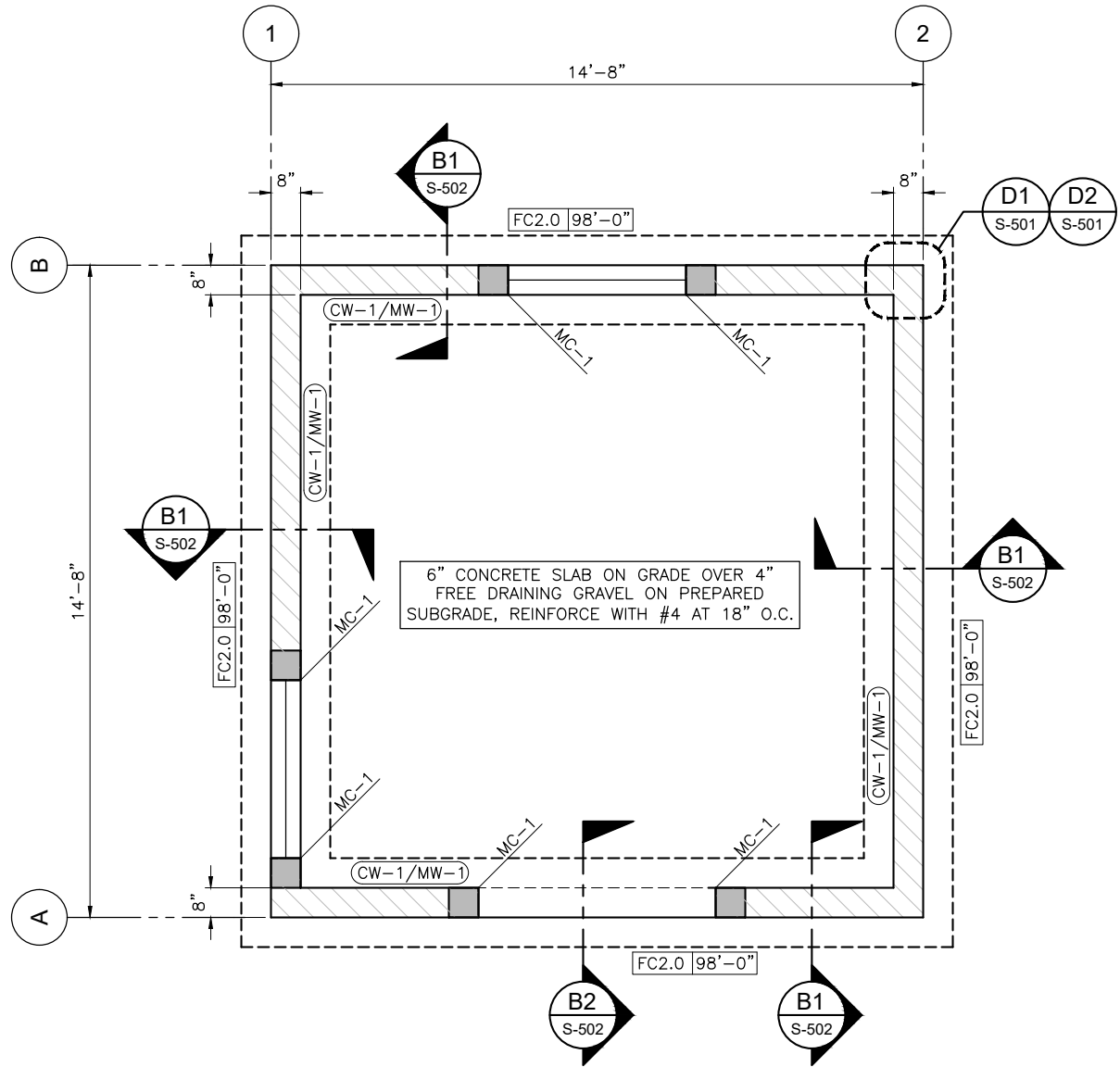
\*\* Does not occur concurrently with top chord live load

D. Design all PFT trusses and bearing attachments for wind uplift, assuming a dead load of 8 psf to resist uplift.

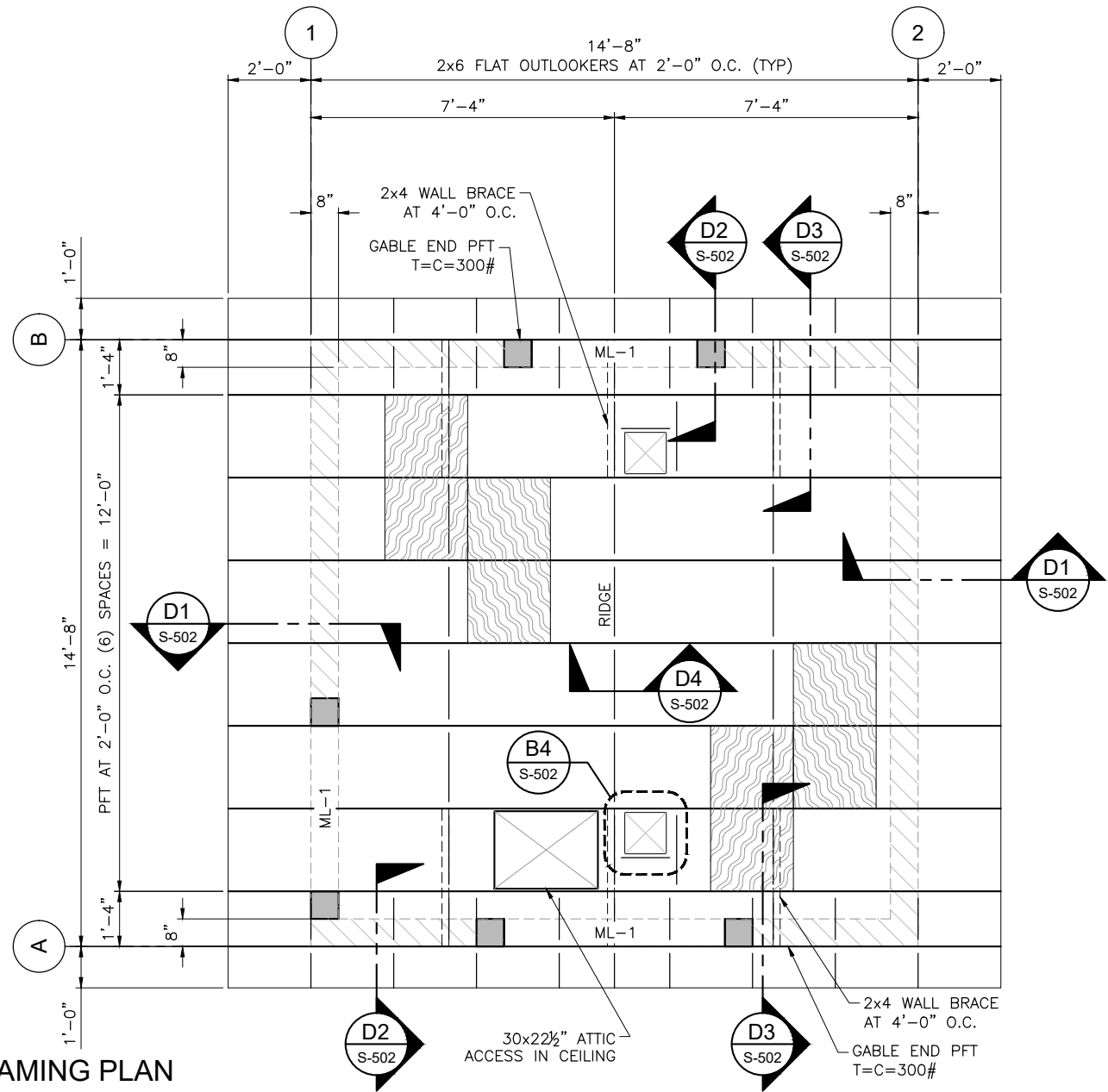
- |               |   |
|---------------|---|
| <b>REVIEW</b> | <b>FILE :</b> CITY STANDARDS_S-001X   |
|               | <b>JUB PROJ. # :</b> ----   |
|               | <b>DRAWN BY:</b> DTJ  |
|               | <b>DESIGN BY:</b> PJT   |
|               | <b>CHECKED BY:</b> PJT  |
|               |  <p>ONE INCH</p> <p>AT FULL SIZE, IF NOT ONE INCH, SCALE ACCORDINGLY</p> |

## TYPICAL LIFT STATION GENERAL STRUCTURAL NOTES

S-002



C1 FOOTING AND FOUNDATION PLAN  
SCALE: 1/2" = 1'-0"



C3 ROOF FRAMING PLAN  
SCALE: 1/2" = 1'-0"

### FOUNDATION AND ROOF FRAMING PLAN NOTES

- SEE ARCHITECTURAL DRAWINGS FOR ALL BUILDING DIMENSIONS. VERIFY FLOOR, CEILING AND SOFFIT ELEVATIONS WITH ARCHITECTURAL ELEVATIONS.
- COORDINATE LOCATION OF ALL PIPE PENETRATIONS AND DUCT PENETRATIONS WITH MECHANICAL AND ELECTRICAL DRAWINGS.
- SEE ELECTRICAL DRAWINGS FOR ALL LIGHTING, ELECTRICAL APPARATUS, AND CONDUIT SIZE AND LOCATION.
- SEE CIVIL DRAWINGS FOR BUILDING LAYOUT AND EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.
- SEE (B2/S-501) FOR COMPACTED STRUCTURAL FILL BENEATH FOOTINGS IF REQUIRED BY SITE SOIL CONDITIONS, SEE GENERAL STRUCTURAL NOTES.
- SEE (B1/S-501) FOR TYPICAL CONCRETE REINFORCING LAP SPLICE SCHEDULE.
- SEE FOOTING AND FOUNDATION DETAILS ON (S-501) FOR BURIED PIPES RUNNING PARALLEL AND PERPENDICULAR TO FOOTINGS.
- SEE (D4/S-501) FOR REINFORCING AROUND MISCELLANEOUS OPENINGS IN MASONRY WALLS.
- SEE (D3/S-501) FOR TERMINATION OF HORIZONTAL WALL REINFORCING AT ENDS OF WALLS AND OPENINGS.
- VERIFY ROOF SLOPES, DRAINS, AND DECK BEARING ELEVATIONS WITH ARCHITECTURAL ELEVATIONS.
- ALL ROOF SHEATHING SHALL HAVE FACE GRAIN PERPENDICULAR TO FRAMING MEMBERS, UNLESS NOTED OTHERWISE. SEE SCHEDULE ON SHEET (S-301) FOR SHEATHING TYPE AND NAILING REQUIREMENTS.
- WEIGHTS AND LOCATIONS OF MECHANICAL EQUIPMENT SHALL BE SUBMITTED TO ENGINEER IN WRITING FOR REVIEW PRIOR TO PLACEMENT OF ROOF FRAMING.
- TRUSS MANUFACTURER TO SUBMIT SHOP DRAWINGS FOR REVIEW OF ALL FRAMING WORK.
- SEE GENERAL STRUCTURAL NOTES ON SHEET (S00-2) FOR DESIGN LOADS OF PREFABRICATED WOOD TRUSSES.
- ALL TRUSS DETAILS, AND TRUSS TO GIRDER TRUSS DETAILS SHALL BE PROVIDED BY THE TRUSS MANUFACTURER.
- TRUSS MANUFACTURER SHALL VERIFY ALL CEILING ELEVATIONS AND SPECIAL CONDITIONS PRIOR TO FABRICATION.
- CONTRACTOR SHALL BE RESPONSIBLE TO PROPERLY BRACE BEAMS, TRUSSES, ETC. AS REQUIRED DURING CONSTRUCTION.
- SEE (B4/S-502) FOR TYPICAL ROOF OPENINGS LESS THAN 24" SQUARE.

### MARKS & SYMBOLS LEGEND

MARK	DESCRIPTION	MARK	DESCRIPTION
FTG ELEV	FOOTING MARK TOP OF FOOTING ELEVATION		MASONRY COLUMN, SEE SCHEDULE ON (S-301)
FC-X	CONTINUOUS FOOTING SEE SCHEDULE ON (S-301)	MC-x	MASONRY COLUMN, SEE SCHEDULE ON (S-301)
	DEPRESSED FOUNDATION WALL, POUR SLAB OVER, SEE (B2/S-502)	ML-x	MASONRY LINTEL, SEE SCHEDULE ON (S-301)
	CONCRETE WALL, SEE SCHEDULE ON (S-301)	T=C=x#	PFT TOP CHORD SHALL BE DESIGNED FOR AN ADDITIONAL LOAD OF "x"# IN TENSION AND COMPRESSION. LOADS PROVIDED ARE AT SERVICE LEVEL.
	MASONRY WALL, SEE SCHEDULE ON (S-301)		ROOF SHEATHING ORIENTATION, SEE SCHEDULE ON (S-301)
CW-x/MW-x	CONCRETE WALL BELOW AND MASONRY WALL ABOVE, SEE SCHEDULES ON (S-301)		

Plot Date: 1/16/2020 1:45 PM Plotted By: Daniel Johnson  
File Path: C:\PROJECTS\JUB\PLANS\CITY STANDARDS\WV\WV CITY STANDARDS S-101.DWG

REVISION			
NO.	DESCRIPTION	BY	DATE


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J-U-B ENGINEERS, INC.

AGENCY

FILE: CITY STANDARDS, S-101X
JUB PROJ. # : ----
DRAWN BY: DTJ
DESIGN BY: PJT
CHECKED BY: PJT
ONE INCH AT FULL SIZE, IF NOT ONE INCH, SCALE ACCORDINGLY

PUBLIC WORKS STANDARDS  
PLAIN CITY CORPORATION

TYPICAL LIFT STATION  
FOOTING AND FOUNDATION PLAN AND ROOF FRAMING PLAN

LAST UPDATED: 2/21/2020  
SHEET NUMBER:  
**S-101**

BAR SIZE	LAP CLASS	f <sub>c</sub> = 3,000 psi		f <sub>c</sub> = 4,000 psi		f <sub>c</sub> = 4,500 psi		f <sub>c</sub> = 5,000 psi	
		CAT. 1	CAT. 2	CAT. 1	CAT. 2	CAT. 1	CAT. 2	CAT. 1	CAT. 2
#4	A	22"	33"	19"	28"	18"	27"	17"	25"
	B	28"	43"	25"	37"	24"	35"	22"	33"
#5	A	27"	41"	24"	36"	23"	34"	21"	32"
	B	36"	53"	31"	46"	30"	44"	28"	41"
#6	A	33"	49"	28"	43"	27"	41"	25"	38"
	B	43"	64"	37"	55"	36"	53"	33"	50"
#7	A	48"	72"	42"	62"	40"	59"	37"	56"
	B	62"	93"	54"	81"	51"	77"	48"	72"
#8	A	55"	82"	47"	71"	45"	68"	42"	64"
	B	71"	106"	61"	92"	58"	88"	55"	83"
#9	A	62"	92"	53"	80"	51"	76"	48"	72"
	B	80"	120"	69"	104"	66"	99"	62"	93"
#10	A	80"	120"	61"	92"	57"	86"	54"	81"
	B	90"	135"	79"	119"	74"	111"	70"	105"

1. FOR GRADE 60 REINFORCING BARS.
2. ALL LAP SPICES SHALL BE CLASS B, UNLESS NOTED OTHERWISE.
3. CATEGORY 1: CLEAR COVER  $\geq$  db & CLR. SPACING  $\geq$  db, AND STIRRUPS OR TIES THROUGHOUT Ld ARE PROVIDED.  
CATEGORY 1: CLEAR COVER  $\geq$  db & CLR. SPACING  $\geq$  2db.  
CATEGORY 2: CLEAR COVER  $<$  db OR CLR. SPACING  $<$  2db.
4. FOR TOP BARS MULTIPLY LAP LENGTH LISTED BY 1.30. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW THE BARS.

**B1** REINFORCING  
SCALE: NOT TO SCALE

[illegible]

AGENCY

## REVIEW

ONE INCH  
AT FULL SIZE, IF NOT ONE  
INCH, SCALE ACCORDINGLY

### TYPICAL LIFT STATION FOOTING AND FOUNDATION DETAILS

S-501



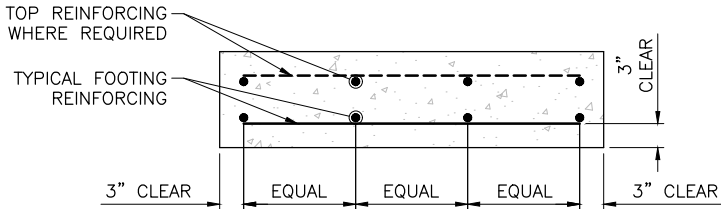
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### CONCRETE FOOTING SCHEDULE

FOOTING MARK	WIDTH	LENGTH	DEPTH	REINFORCING CROSSWISE				REINFORCING LENGTHWISE				REMARKS
				NO.	SIZE	LENGTH	SPACING	NO.	SIZE	LENGTH	SPACING	
FC2.0	2'-0"	CONT	12"	-	-	-	-	3	#4	CONT	EQ	

#### CONCRETE FOOTING NOTES:

1. PLACE ALL FOOTING REINFORCING IN BOTTOM OF FOOTING WITH 3" CLEAR CONCRETE COVER UNLESS OTHERWISE NOTED.
2. TOP REINFORCING, WHERE SPECIFIED, SHALL BE PLACED IN THE TOP OF THE FOOTING WITH 2" MINIMUM CONCRETE COVER.
3. IF FOOTINGS ARE EARTH FORMED, FOOTING WIDTH AND LENGTH SHALL BE 6" WIDER AND LONGER THAN SCHEDULED.
4. NOT ALL FOOTINGS ARE USED, SEE FOUNDATION PLAN FOR FOOTING MARKS.
5. RUN CONTINUOUS BARS IN "FC" FOOTING THROUGH INTERSECTED "FS" FOOTINGS.
6. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.
7. CENTER SPOT FOOTINGS AT COLUMN LOCATIONS.



TYPICAL FOOTING SECTION

B1

### CONCRETE FOOTING SCHEDULE

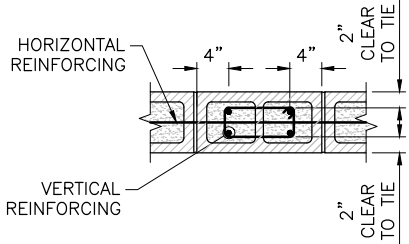
SCALE: NOT TO SCALE

### MASONRY COLUMN SCHEDULE

COLUMN MARK	COLUMN SIZE	REINFORCING			REMARKS
		VERTICAL	TIES	CONFIGURATION	
MC-1	8"x8"	(1) #5	NONE	•	

#### MASONRY COLUMN NOTES:

1. THE CENTERLINE OF VERTICAL BARS SHALL BE LOCATED 2 1/2" FROM THE FACE OF THE MASONRY. HORIZONTAL WALL REINFORCING SHALL BE LOCATED TO THE INSIDE OF THE VERTICAL BARS.
2. UNLESS NOTED OTHERWISE, VERTICAL REINFORCING AND TIES SHALL EXTEND TO FULL WALL HEIGHT.
3. VERTICAL MASONRY COLUMN REINFORCING SHALL EXTEND INTO THE FOOTING AND TERMINATE WITH A STANDARD 90° HOOK.
4. IN CONCRETE FOUNDATION WALLS, VERTICAL MASONRY COLUMN REINFORCING SHALL BE TIED WITH #3 TIES AT THE SAME SPACING AND CONFIGURATION AS MASONRY COLUMNS ABOVE.
5. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.



SCHEMATIC MASONRY COLUMN CONFIGURATION

D1

### MASONRY COLUMN SCHEDULE

SCALE: NOT TO SCALE

### CONCRETE WALL SCHEDULE

WALL MARK	THICKNESS	REINFORCING		WALL TYPE	REMARKS
		VERTICAL	HORIZONTAL		
CW-1	8"	(1) #5 AT 16"oc	(1) #4 AT 12"oc	A	

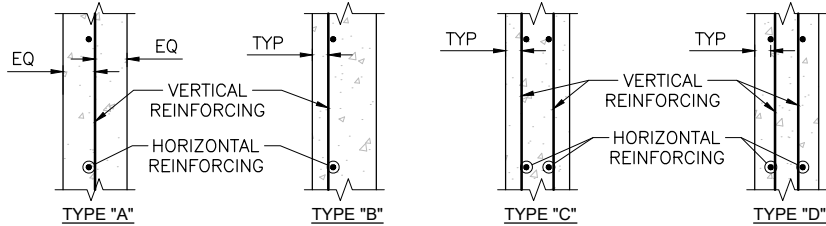
#### CONCRETE WALL NOTES:

1. SEE GENERAL STRUCTURAL NOTES FOR REQUIREMENTS NOT SHOWN IN SCHEDULE.
2. CONCRETE WALLS NOT DESIGNATED ON THE PLANS SHALL BE REINFORCED AS FOLLOWS:

THICKNESS	VERTICAL REINFORCING	HORIZONTAL REINFORCING
6"	#4 AT 18"oc	#4 AT 16"oc
8"	#4 AT 18"oc	#4 AT 12"oc
10"	#4 AT 16"oc	#5 AT 15"oc
12"	#4 AT 18"oc EACH FACE	#4 AT 16"oc EACH FACE

- 2.1. PLACE STEEL IN THE CENTER OF THE WALL (EXCEPT TYPE "B" AND RETAINING WALLS). WALLS THICKER THAN 10" SHALL HAVE TWO CURTAINS OF REINFORCEMENT (PLACED NEAR EACH FACE OF THE WALL) UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS.
- 2.2. FOR WALLS 10" AND THINNER (1) #7 (OR (2) #5) x CONTINUOUS HORIZONTAL BAR SHALL BE PLACED AT THE BOTTOM OF THE WALL (NEAR THE FOOTING) AT EACH FLOOR LEVEL, AT THE ROOF LEVEL, AND AT THE TOP OF WALL. FOR WALLS THICKER THAN 10" (2) #5 BARS SHALL BE PLACED AT THESE LOCATIONS.

#### CONCRETE WALL PLACEMENT TYPES:



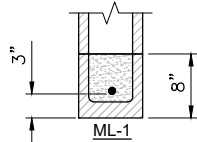
B2

### CONCRETE WALL SCHEDULE

SCALE: NOT TO SCALE

### MASONRY LINTEL SCHEDULE

LINTEL MARK	LINTEL DEPTH	LINTEL SPAN MAXIMUM	REINFORCING	
			HORIZONTAL	STIRRUPS
ML-1	8"	5'-4"	(1) #5 BAR CONT	NONE



#### MASONRY LINTEL NOTES:

1. LINTEL WIDTH AND MATERIAL TYPE SHALL BE THE SAME AS THE WALL IN WHICH THE LINTEL IS CONSTRUCTED.
2. GROUT MASONRY LINTELS MONOLITHICALLY WITH THE SUPPORT WALL OR COLUMN AT EACH END.
3. MASONRY LINTEL ML-1 SHALL BE USED OVER OPENINGS IN MASONRY WALLS WHEN A SPECIFIC MASONRY LINTEL IS NOT OTHERWISE SPECIFIED. WHEN A LINTEL IS SPECIFIED ON THE PLANS, THE MAXIMUM SPAN AS NOTED IN THIS SCHEDULE SHALL NOT APPLY. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SPECIFIED ON THE PLANS WHICH HAVE A SPAN GREATER THAN 12'-0".
4. EXTEND ALL HORIZONTAL REINFORCING 48 BAR DIAMETERS MINIMUM BEYOND THE EDGE OF ALL OPENINGS. IF HORIZONTAL REINFORCING CANNOT EXTEND 48 BAR DIAMETERS BEYOND EDGE OF OPENING, PROVIDE 90° STANDARD HOOK.
5. SPLICE TOP BARS AT MID-SPAN OF LINTEL ONLY AND BOTTOM BARS OVER SUPPORTS ONLY.
6. HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
7. DOWEL VERTICAL REINFORCING OF WALL ABOVE LINTEL INTO THE FULL DEPTH OF LINTEL OR 48 BAR DIAMETERS, WHICHEVER IS LESS.
8. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.

D2

### MASONRY LINTEL SCHEDULE

SCALE: NOT TO SCALE

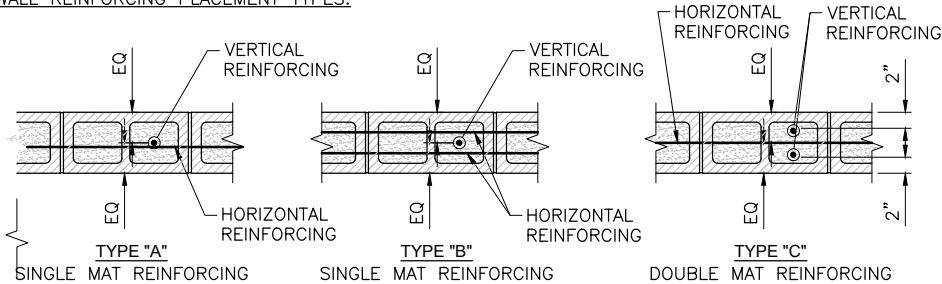
### MASONRY WALL SCHEDULE

WALL MARK	THICKNESS	fm (psi)	SOLID GROUT	REINFORCING			SPECIAL INSPECTION
				VERTICAL	HORIZONTAL	TYPE	
MW-1	8"	1500	NO	(1) #5 AT 32"oc	(1) #5 AT 32"oc	A	YES

#### MASONRY WALL NOTES:

1. DO NOT SOLID GROUT WALLS UNLESS NOTED OTHERWISE.
2. INSTALL LOOSE FILL INSULATION IN ALL UNGROUTED CELLS WHERE NOTED.
3. ALL MASONRY BELOW GRADE SHALL BE SOLID GROUTED.
4. VERTICAL REINFORCING SHALL BE CENTERED IN THE WALL UNLESS NOTED OTHERWISE.
5. (1) VERTICAL BARS MINIMUM AT ALL CORNERS AND END OF WALLS.
6. HORIZONTAL WALL REINFORCING SHALL BE PLACED BETWEEN VERTICAL MASONRY COLUMN REINFORCING BARS.
7. HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
8. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.

#### WALL REINFORCING PLACEMENT TYPES:



B3

### MASONRY WALL SCHEDULE

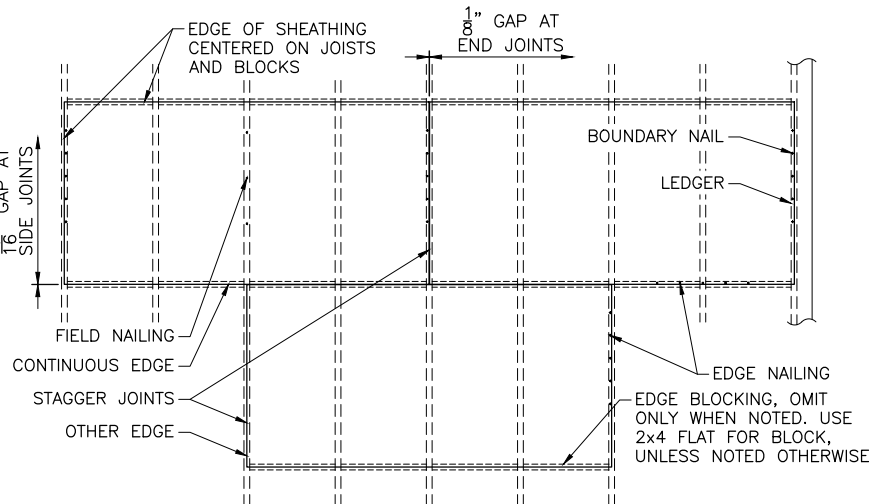
SCALE: NOT TO SCALE

### ROOF SHEATHING SCHEDULE

LOCATION	WOOD SHEATHING THICKNESS	NAIL SIZE	EDGE NAIL		FIELD NAIL	BOUNDARY NAIL	EDGE BLOCK
			CONT EDGE	OTHER EDGE			
TYPICAL	7/16" (24/16)	8d	6"oc	6"oc	12"oc	6"oc	NO

#### ROOF SHEATHING NOTES:

1. MINIMUM NAIL PENETRATION INTO FRAMING: 8d-1 1/2", 10d-1 3/8"
2. USE COMMON NAILS (8d DIAMETER=0.131", 10d DIAMETER=0.148")
3. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.



D3

### ROOF SHEATHING SCHEDULE

SCALE: NOT TO SCALE

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#### REVISION

NO.	DESCRIPTION	BY	DATE

J-U-B ENGINEERS, INC.

466 North 900 West  
Kaysville, Utah 84037

Phone: 801.547.0393  
Fax: 801.547.0397  
www.jub.com



J-U-B ENGINEERS, INC.

AGENCY

FILE: CITY STANDARDS, S-101X

JUB PROJ. # : ---

DRAWN BY: DTJ

DESIGN BY: PJT

CHECKED BY: PJT

ONE INCH  
AT FULL SIZE, IF NOT ONE  
INCH, SCALE ACCORDINGLY

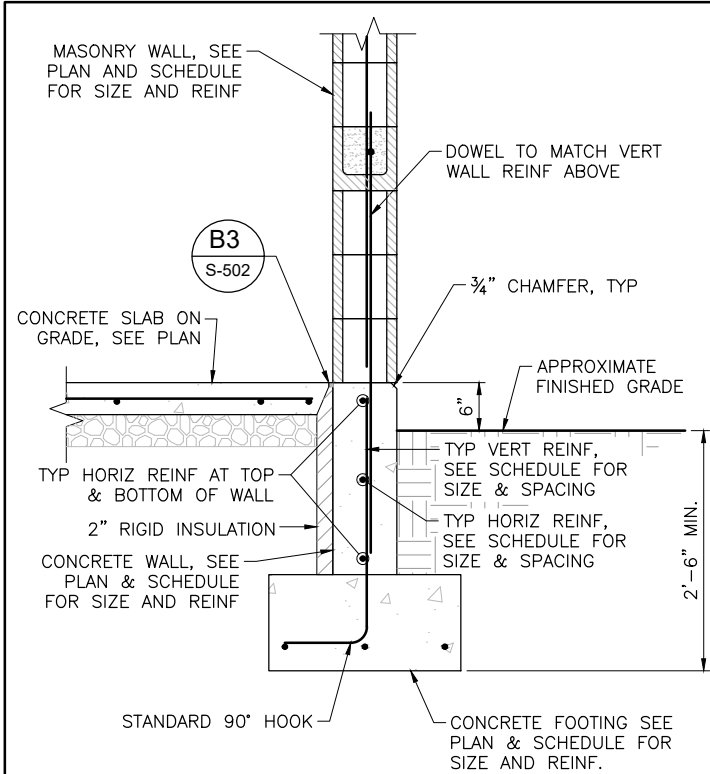
PUBLIC WORKS STANDARDS  
PLAIN CITY CORPORATION

TYPICAL LIFT STATION  
SCHEDULES

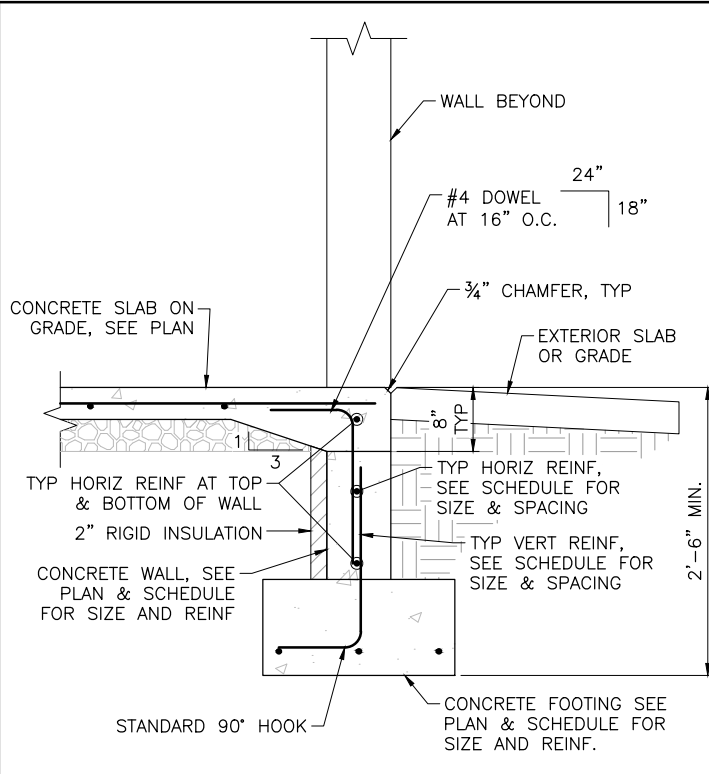
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SHEET NUMBER:

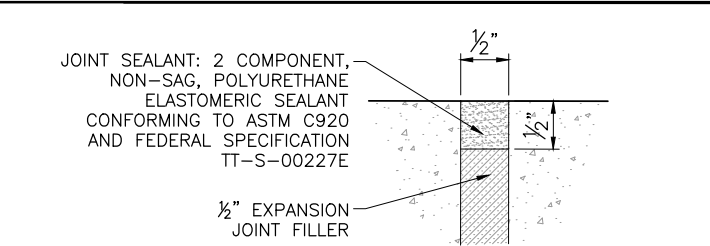
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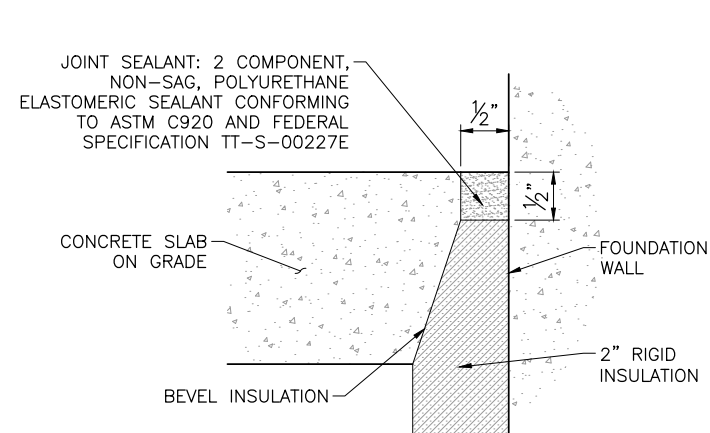
**B3** CMU WALL BEARING AT EXTERIOR CONCRETE FOOTING  
SCALE: 1" = 1'-0"



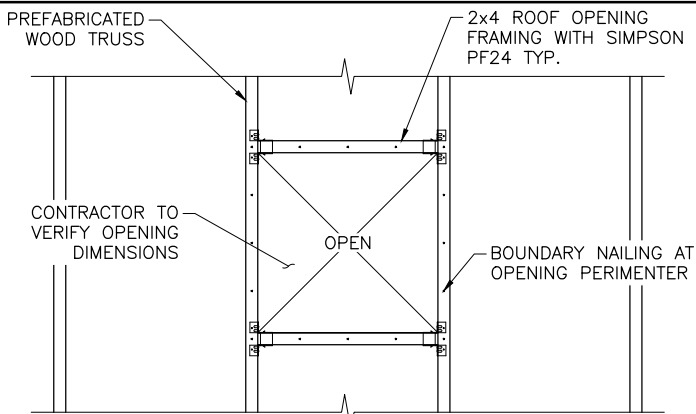
**B2** CONCRETE FOUNDATION WALL AT OPENING  
SCALE: 1" = 1'-0"



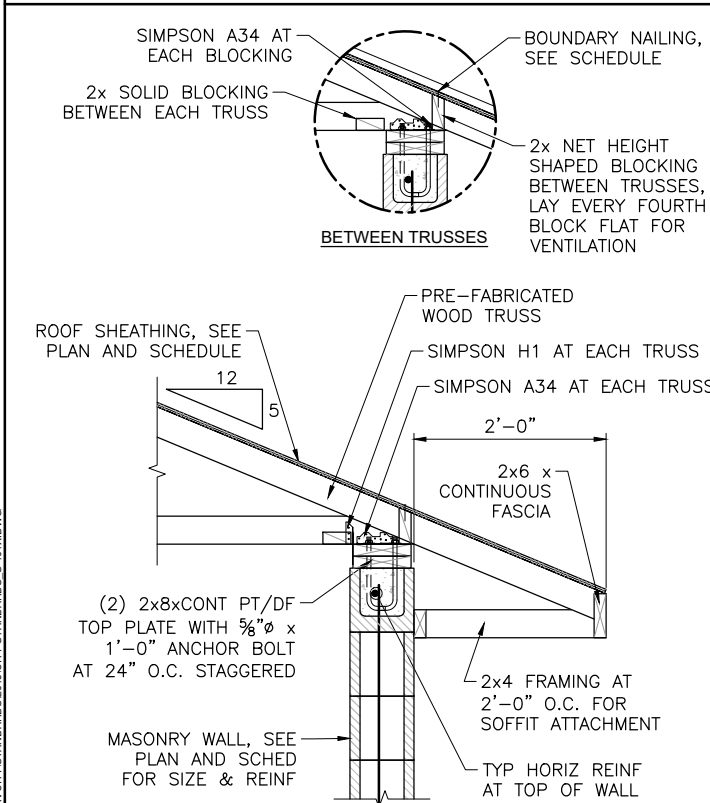
**A3** JOINT SEALANT DETAIL  
SCALE: 6" = 1'-0"



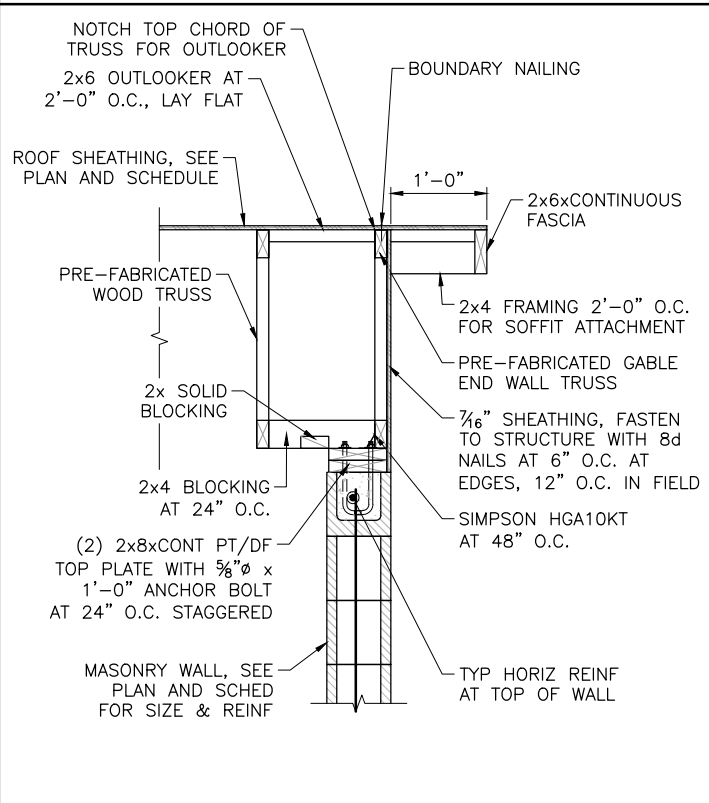
**B3** JOINT INSULATION DETAIL  
SCALE: 6" = 1'-0"



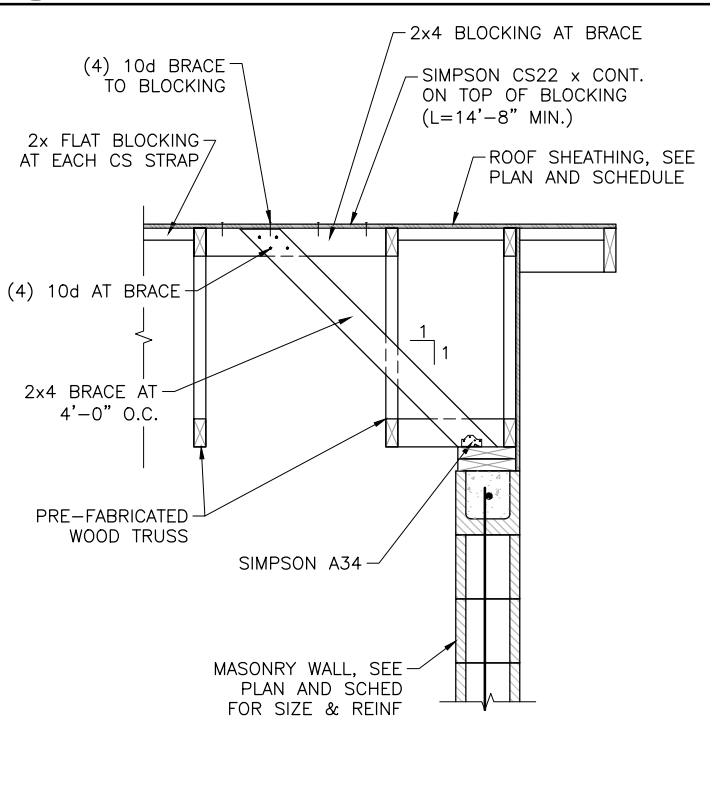
**B4** TYP WOOD ROOF OPENING DETAIL (OPENING LESS THAN 24" SQUARE)  
SCALE: 1" = 1'-0"



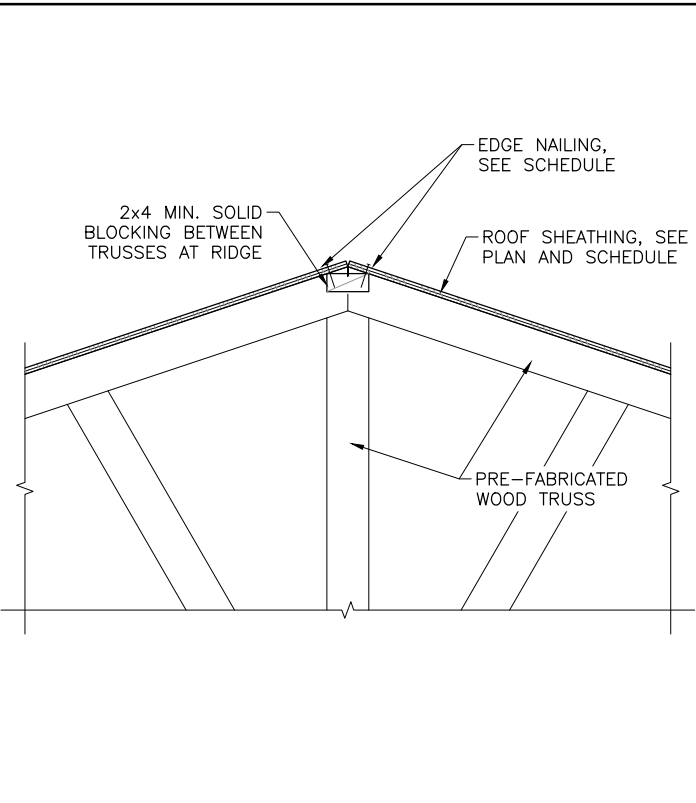
**D1** PFT BEARING AT CMU WALL  
SCALE: 1" = 1'-0"



**D2** GABLE END PFT AT CMU WALL  
SCALE: 1" = 1'-0"



**D3** CMU WALL BRACE AT GABLE END  
SCALE: 1" = 1'-0"



**D4** RIDGE BLOCKING  
SCALE: 1 1/2" = 1'-0"

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**JUB**  
J-U-B ENGINEERS, INC.

**AGENCY**

**REVIEW**

FILE: CITY STANDARDS, S-101X  
JUB PROJ. # : ----  
DRAWN BY: DTJ  
DESIGN BY: PJT  
CHECKED BY: PJT

ONE INCH  
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INCH, SCALE ACCORDINGLY

**PUBLIC WORKS STANDARDS  
PLAIN CITY CORPORATION**

**TYPICAL LIFT STATION  
FOOTINGS AND FOUNDATION DETAILS AND ROOF FRAMING DETAILS**

LAST UPDATED: 2/21/2020  
SHEET NUMBER:  
**S-502**